GEOTECHNICAL INVESTIGATION REPORT

PRELIMINARY

EIGHT POINT WIND FARM WIND TURBINES AND TRANSMISSION LINE STEUBEN COUNTY

NEW YORK

PREPARED FOR

EIGHT POINT WIND LLC

700 UNIVERSE BLVD. JUNO BEACH, FL 33408



PREPARED BY:

KENNEY GEOTECHNICAL ENGINEERING SERVICES PLLC

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INTRODUCTION

This report presents the findings of the geotechnical investigation performed at accessible wind turbine and transmission line locations during the first mobilization for the proposed Eight Points Wind Farm and Transmission Line. This report presents an analysis of the available geotechnical data and recommendations for the proposed construction. This report has been prepared for NextEra Energy Resources, Inc. (NextEra).

No environmental services are included in this study. No conclusions have been drawn regarding environmental conditions of the site, potential contaminants, potential special treatment or disposal of site materials, or other environmental considerations.

AUTHORIZATION

Our services for this project were authorized by Eight Point Wind LLC Purchase Order 2000239122 dated May 12, 2017. Our scope of work was developed by Sargent and Lundy LLC in the document entitled "Eight Point Wind Substation Geotechnical Investigation Work & Specification Detail", Revision A, dated July 13, 2017.

PROJECT DESCRIPTION

The proposed wind farm and transmission line will be constructed over a large portion of Steuben County, New York stretching from the Pennsylvania state line in the south to Hornell in the north (see Figure 1). It is our understanding that the proposed project will consist of twenty-seven 3.43MW GE wind turbine generators, four 2.3 MW GE wind turbine generators, and a transmission line approximately 16.5 miles in length. We further understand that the wind turbine foundations are assumed to consist of typical spread (inverted T) foundations. Transmission poles are assumed to be steel monopole structures, direct-embedded, with an estimated butt diameter of 36 to 60 inches typical. Tangent structures will utilize braced post assemblies in a triangular configuration. Light angle un-guyed structures will have up to an eight degree line deflection. Medium angle structures will have line deflections of ten to forty degree and will be guyed on bisector with one or two guys per phase and on for the shield wires. Dead ends with a deflection angle greater than 40 degrees will be guyed with two guys per phase and one per shield wire.

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AREA PHYSIOGRAPHY

The project site is located within south central part of New York State in the glaciated portion of the Allegheny Plateau physiographic section of the Appalachian Plateau physiographic province as seen in Appendix A. Major topographic and geologic features in this area were formed during the last glacial advance and retreat, which ended approximately 12,000 years ago. The site is located just south of the most recent glacial maximum advance.

The project site is a hilly highland area dissected by creeks and rivers. Publically available information suggest soils on most of the uplands formed in glacial till. The valley sections are primarily glacial outwash or alluvial sediments. The following section from the USDA Soil Survey of Steuben County provides a good generalization of typical subsurface conditions.



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The proposed wind turbines will be constructed on upland areas. The transmission line will encounter a wide variation of soil types as it crosses upland areas, slopes, and valley bottoms.

GEOLOGIC CONDITIONS

Publically available geologic mapping suggests that Oswayo and Cattaraugus formation shale, siltstone and sandstone underlies soils at the site. The bedrock may be located within close proximity to the ground surface in many upland areas. The shale and sandstone is typically horizontally bedded with near-vertical orthogonal stress fractures.

The transition to bedrock from the overlying glacial till is chaotic in the project area. During glaciation large sections of rock were moved short distances or not at all but were detached from the bedrock below. When they weathered, they settled and tilted. Sometimes just the layers that are chemically weaker weather (iron rich, mica rich, feldspars or layers that are cemented with calcite). The rock pops open along bedding planes and clay rich minerals swell, mechanically breaking down the rock. This allows more surface area for chemical weathering creating void and allowing sediment from above to wash into the open voids over the past 30-40 thousand years. The result is that soil zones are encountered below what appears to be solid bedrock.

Groundwater aquifers are located in the glacial outwash in the valley areas. Upland areas typically do not have a true water table above a depth of approximately 70 or 80 feet. However upland areas are subject to perched groundwater conditions during wetter periods as infiltrating water becomes trapped within the soil above undisturbed clayey glacial till and bedrock.

Oil and natural gas wells are located in the vicinity of the project site. The local oil and gas wells typically draw from bedrock formations located 4000 to 5000 feet below the ground surface.

SUBSURFACE EXPLORATION

The subsurface exploration program for this phase of investigation was limited to those sites that could be accessed without clearing trees. A total of 24 borings were advanced at wind turbine locations and 9 borings were advanced at transmission line structures for this preliminary report. Table 1 presents a summary of the borings performed to date. The scope and specifications for the drilling program were established by NextEra Energy and Sargent & Lundy, the wind turbine and transmission line designers, respectively. The test boring location, depth, sampling,





and refusal criteria are summarized in Table 2.

The test borings were advanced to depths of up to 42.1 feet using CME 550 and Geoprobe 7822DT all-terrain drill rigs equipped with an automatic sampling hammer, hollow stem augers, and NQ double-tube diamond core barrels. Standard penetration testing (ASTM D1586) was performed during advancement of the augers through overburden. Coring was performed after auger refusal occurred.

Soil samples obtained during the subsurface investigation were classified by a Geotechnical Engineer using the Unified Soil Classification System. Boring logs documenting the subsurface conditions encountered are attached. The boring logs and related information depict subsurface conditions only at the specific locations and times indicated. Subsurface conditions and water levels at other locations may differ from conditions at the locations where sampling was conducted. The passage of time also may result in changes in the conditions interpreted to exist at the locations where sampling was performed.

SUBSURFACE CONDITIONS ENCOUNTERED

The following interpretation of subsurface conditions is based on our review of the recovered samples, the boring logs, drilling observations, laboratory testing results, and our professional experience

The four primary strata encountered at wind turbine locations during the subsurface investigation included:

- Stratum A- Glaciofluvial Soil
- Stratum B Glacial Till
- Stratum C Transition Zone Rock, and
- Stratum D Bedrock

Stratum A – Glaciofluvial Soil was encountered at the ground surface at some of the transmission line locations and extended to depths of up to 6 feet. This stratum was primarily composed of loose to medium dense silty sand with gravel. Standard Penetration Testing "N" values in this stratum were typically less than 20 blows per foot.

Stratum B – Glacial Till was typically encountered from the ground surface to depths of up to 25 feet and consisted of very stiff to hard silt and lean clay with sand, gravel, cobbles and boulders. With the exception of the uppermost two feet, which was softened by weathering, Standard Penetration Testing "N" values in this stratum typically ranged between 20 to 100 blows per foot.



Stratum C – Transition Zone Rock was encountered at the interface of Strata B and D. This stratum consisted of bedrock that had been altered by glaciation and/or weathering. In some areas the Transition Zone Rock consisted of large slabs of detached competent bedrock with interbedded soil zones. In other areas the Transition Zone Rock consisted of highly weathered bedrock. The detached bedrock typically consisted of the local sandstone and siltstone. The interbedded soil zones typically consisted of red clay with angular green sandstone fragments the size of gravel. The red clay was derived from the weathering of the local red shale. This stratum typically had to be cored for sample recovery. At locations where augers could penetrate into this stratum the Standard Penetration Testing "N" values typically exceeded 100 blow per foot.

Stratum D- Bedrock was encountered below Stratum C. The bedrock encountered consisted of fresh sandstone, siltstone, conglomerate, shale and slate. This stratum was sampled by coring. The recovered bedrock core were typically fresh, medium hard to hard rock with horizontal bedding planes. Photographs of the rock cores are included in the Appendix.

GROUNDWATER CONDITIONS ENCOUNTERED

Groundwater observations during this phase of investigation were limited to water measurement in the augers. Piezometer installation, and long term groundwater observations, were not authorized at this time.

Water was present in the borehole following drilling at some of the locations where rock coring was performed. However, water was introduced into the borehole during coring operations and it is not possible to distinguish between water remaining from coring operations and possible perched groundwater at the transition zone/bedrock interface.

It if our opinion that a true groundwater table was not encountered during the subsurface investigation. Available public data indicates the groundwater table is located in bedrock approximately 70 feet or more below the ground surface in the project area. The depth of groundwater will vary with changes in precipitation patterns and other factors. No long term groundwater monitoring was performed as part of this study.

Glacial till samples recovered during drilling typically exhibited low moisture content and it did not appear that a perched groundwater table was present within the till at the time of drilling. However, the glacial till will have a low in-situ permeability and it is possible that water will collect in backfill surrounded by the

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till.

LABORATORY TESTING

Laboratory testing was performed upon samples obtained during the geotechnical investigation and included:

- Moisture content (ASTM D2166),
- Particle size analysis (ASTM D422),
- Compaction characteristics (ASTM D698 or ASTM D1557)
- Corrosion series (chloride, sulfate, and pH),
- Unconfined compressive strength (ASTM D2166))
- Atterberg limits (ASTM D4318), and
- Unconfined compressive strength and unit weight (ASTM D2928) for intact rock core specimens.

Test results are attached to this report.

SEISMIC CONDITIONS

The project site is located in an area of relatively low seismic activity. The USGS Seismic Hazards database indicates a 0.74% chance of a magnitude 5.0 earthquake occurring in the next 50 years in the project area. The site has a dense soil cover and will not provide significant amplification of seismic waves.

Geophysical surveys are part of the overall scope of services but were not authorized for this phase of the investigation and no site-specific shear wave velocity data is available. Based upon correlations with Standard Penetration Testing "N" values and New York State Building Code guidelines the available data suggests that **Site Class C** is appropriate. The estimated design spectral response acceleration parameters are $S_{DS} = .101g$ and $S_{D1} = .060g$. Liquefaction, surface rupture from faulting or lateral spreading is estimated to have a low probability of occurrence given the soil conditions encountered and typical regional seismicity.

SOIL CHEMISTRY

Laboratory corrosion series testing was performed on six site samples. Results are as follows:

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			Sulfates	Chlorides	
Boring	Depth	\mathbf{pH}	(ppm)	(ppm)	
T-4	2-4'	8.1	<50	<50	
Т-9	0-2'	6.2	<50	<50	
T-18	4-6'	7	<50	<50	
T-24	4-6'	6.7	<50	<50	
T-29	-29 2-4' 6.1		<50	<50	
ALT-2	4-6'	5.8	<50	<100	

In general, a pH of less than 3.5, a chloride concentration greater than 500 parts per million (ppm), or a sulfate concentration greater than 2000 ppm is considered to be indicative of a corrosive environment for most structures. Based on the test results it appears that standard Type I/II cement may be utilized on this project.

GEOTECHNICAL ANALYSIS

<u>Overview</u>

Based on the subsurface conditions encountered during the investigation performed to date, it appears that the primary geotechnical issues will be:

- Excavation of the glacial till, transition zone rock and bedrock;
- Possible deterioration of the glacial till and transition zone rock upon excavation and exposure to the elements and construction traffic.

The glacial till encountered in the area typically consists of a binder of hard, lowplasticity silty clay that encapsulates particles ranging in size from fine sand to boulders the size of automobiles. The transition zone rock appears to include large slabs of intact medium hard to hard bedrock with weathered seams that are infilled with soil.

The glacial till, transition zone rock and bedrock will provide high bearing strength and good short term excavation stability if left undisturbed. However, the strength of the glacial till and transition zone rock strata will deteriorate if they are allowed to saturate or if they are disturbed by over-excavation. The stability of slopes and excavations in these strata will decrease over time. Typically permanent slopes in the glacial till are graded no more steeply than 33% (18.4 degrees) unless they are reinforced.

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Turbine Foundations

The available information suggests that the wind turbine foundations will be underlain by hard glacial till, transition zone rock with soil infill, and bedrock. It is therefore anticipated that the turbines can utilize a gravity shallow foundation system or a Patrick & Henderson Tensionless Pier (PHTP) foundation system. Design frost depth is four feet in the project area, and foundations must bear below this depth to prevent movement due to frost heave.

The glacial till typically provides high bearing strength and good short term excavation stability if it is left undisturbed. The glacial till contains a significant percentage of silt and clay and loses strength rapidly if saturated and subjected to dynamic loading such as that imparted by construction equipment.

Assuming the foundation excavations are properly managed during construction, a allowable bearing pressure of 5,000 pounds per square foot is appropriate for shallow foundations bearing on undisturbed glacial till. An allowable bearing pressure of 6,500 pounds per square foot is estimated for foundations bearing on Stratum C-Transition Zone Rock materials. An allowable bearing pressure of 10,000 pounds per square foot is estimated for foundations bearing on Stratum D – Bedrock. The turbine foundations excavations must be reviewed by geotechnical personnel to verify these allowable bearing pressures are appropriate during construction.

Settlement estimates will require more detailed information concerning turbine loading and will be prepared by others after the geotechnical investigation is completed. Based upon the borings performed to date, the following geotechnical parameters are suggested for preliminary turbine foundation design:

Stratum	Avg Thickness (ft)	Unit Weight (pcf)	Tota ¢	Total Stress I Cohesion c \$ (psf)		ve Stress c' (psf)	k (pci)	ε50	Allow. Skin Friction (ksf)	Кр	
Stratum B - Glacial Till	15	135	0	2500	34	300	1000	0.005	1	9	
Stratum C - Transition Zone	30	140			32	2500	1000	0.005	6	18	
Stratum D - Bedrock	n/a	145			36	3500	2000	0.004	10	32	



Transmission Line Monopoles

Drilled shafts and direct embedment monopoles set in grout or concrete can be utilized to support the transmission line structures, although it will be necessary to core through boulders, detached rock slabs, and hard bedrock. It is assumed the drilled shafts will be constructed using temporary steel casing to support the drilled hole above competent rock. Loose material should be removed from the bottom of the drilled shaft prior to the placement of concrete. Dewatering should be performed prior to concrete placement if more than 2 feet of water accumulates in the bottom of the shaft. Concrete should be placed in the shafts the same working day that drilling is completed. A dropchute that extends at least 75% of the length of the drilled shaft should be utilized during concrete placement. The top of the fluid concrete placement must be maintained at least 3 feet above the bottom of the temporary casing as it is being extracted during concrete placement. Reinforcing cages should be supported at the ground surface until the concrete adequately hardens.

Suggested geotechnical design parameters for each of the transmission line sites is presented in a table attached to the end of this report.

The estimated parameters assume proper management of geomaterials during construction.

Access Roads and Earthwork

The glacial soils encountered contain significant percentages of silt and clay, may be saturated during wetter periods, and will be subjected to freeze-thaw cycles during the winter months. The design of unpaved access roads for this project will require careful consideration of these factors. The glacial till loses strength rapidly if saturated and subjected to dynamic loading such as that imparted by construction equipment. Freeze-thaw action will significantly deteriorate access road subgrades composed of glacial till. Construction access roads will deteriorate rapidly without a gravel base and geosynthetic reinforcement. Proper drainage of roads and excavations will be essential to maintaining their stability.

The glacial soils have generally acceptable engineering properties for the proposed construction. However, moisture control of soils will be critical during earthwork for this project. Glacial soils that are exposed to the elements will saturate and lose strength rapidly. Cut slopes in glacial soils will lose strength over time. Cut or fill slopes using the glacial till should be designed for a final grade no greater than 3H:1V (33%).



We anticipate that temporary excavations can be open cut. Excavation at the site will require large hydraulic excavators with rock teeth. Large hoe rams may be necessary to remove boulders and rock slabs. Smaller excavations in bedrock could require blasting.

Temporary excavation slopes must be evaluated by the Contractor's on-site Responsible Person. We anticipate soil above the groundwater table will be classified as Type B. Type B materials must be graded to slopes no greater than 1:1 (horizontal to vertical). The Contractors on-site Responsible Person should periodically review excavations for signs of movement or distress. Excavation sidewalls should be periodically raked to remove loose particles.

Earthwork must be performed using methods that will result in a stable excavations and fills. Typical temporary earthwork measures such as temporary drainage swales, stabilized haul roads, and the use of protective layers of crushed stone can be employed at this site. It is recommended that earthwork is observed by geotechnical personnel to ensure that all organic material is removed from beneath structures and roadways. Additional recommendations are as follows:

- Strip existing topsoil, pavement, roots and organics from all areas that will receive new construction to establish subgrade.
- Proof-roll exposed slab-on-grade subgrade with a fully loaded dump truck, or accepted alternative equipment, under the observation of geotechnical personnel. Areas that rut, weave, or deflect should be over-excavated and replaced with compacted structural fill (see below for structural fill characteristic requirements).
- Utilize structural fill to raise site grades to the desired elevation. Structural fill should consist of imported granular material conforming to NYSDOT Subbase Course (2" minus), NYSDOT Item 4 or 304.12 aggregate, or approved equal.
- Field moisture contents for structural fill should be maintained within 2 percentage points of the optimum moisture content established by laboratory testing to provide adequate compaction. All fill should be placed in level lifts having a loose thickness no greater than 12 inches and should be compacted with vibratory rollers to at least the following minimum percentages of the Modified Proctor (ASTM D-1557) maximum dry density:

•	Below foundations:	95%
•	Beneath slab-on-grade or access roads:	95%
•	Utility trench backfill:	95%
•	Beneath landscape areas:	90%



• Beneath sidewalks and exterior slabs: 95%

Bulk samples of proposed structural fill materials should be delivered to our testing laboratory at least two weeks prior to the initiation of earthwork. In-place density testing should be performed at a frequency of one test per 500 square feet per lift in open areas and one test per 25 feet per lift in trenches.

- If the structure is to be constructed during the winter months, adequate frost cover and protection must be provided. Earthwork cannot be performed with frozen material.
- Permanent slopes should be graded no steeper than 3 horizontal: 1 vertical.
- In utility trenches, or other confined areas, small compaction equipment may be necessary such as a vibratory plate, jumping jack or walk-behind vibratory roller. In-place density testing should be performed at a frequency of one test per 25 feet per lift in trenches. Utility trench fill should be placed in level lifts no greater than 8 inches in thickness and should be compacted to at least 95% the Modified Proctor (ASTM D-1557) maximum dry density. Structural fill should consist of imported granular material such as NYSDOT Subbase (2" minus) or approved equal. Adequate frost cover and protection must be provided during winter weather construction. Earthwork cannot be performed with frozen material.

Groundwater Seepage and Management

Groundwater was not encountered during drilling within the anticipated depths of excavation. As previously noted, the glacial till has a low in-situ permeability and a bathtub effect may occur around structures placed and backfilled within the glacial till. We anticipate that temporary dewatering measures, such as sump and pump methods, will be adequate to control groundwater and allow construction to proceed "in the dry". All dewatering discharge should be to a temporary dewatering basin constructed consistent with NYDEC construction stormwater and/or dewatering treatment Best Management Practices (BMPs).

LOCAL SOURCES OF CONSTRUCTION MATERIALS

Concrete for foundations and aggregate for unpaved roadways can be obtained in Allegheny and Steuben Counties. Ready-mix concrete is available in a variety of strengths from:



- Hanson Aggregates, Alfred Station, NY (607) 276-5881
- Coots Concrete, Bath, NY (607) 776-3966

Gravel and crushed stone can also be obtained from these producers. The subbase material most commonly specified in this area is referred to as "Item 4" sand and gravel. This is a crushed gravel or crushed rock product engineered to meet the the gradation NYS DOT Type 2 subbase.

LIMITATIONS

The recommendations presented in this report are predicated on the performance of construction observation and testing by qualified geotechnical personnel. We request continued involvement with this project so that we may assess subsurface conditions exposed during construction to determine if modifications to our recommendations are necessary.

REFERENCES

"Surficial Geologic Map of New York – Central New York Sheet", New York State Museum, 1986.

"Bedrock Map of New York – Central New York Sheet", New York State Museum, 1970.

"Soil Survey of Steuben County", USDA, 1973

"FAD Tools – FAD 5.1 Users Guide", Revision 0, December 2015

"Hazard Analysis Report, Steuben County, New York", Steuben County Office of Emergency Services, April 1, 2014



tructure Name (old)	tructure Name (new)	Structure Type	Deg	Latitude Min Sec Dir		Dir	Long Deg Min		gitude Sec Dir		Horizontal Datum	Site Elevation (AMSL Feet)	Vertical Datum	structure Height (AGL Feet)	
S	S													Ś	
1	1	Wind Turbine	42	08	01.107	N	//	43	32.052	E	NAD83	2258	NAVD88	586	
2	2	Wind Turbine	42	08	04.347	IN N	77	42	33.635	E	NAD83	2359		586	
3	3	Wind Turbine	42	07	40.200	IN N	77	43	45.000	с с	NAD83	2300		580	
4 E	4 F	Wind Turbine	42	00	15 607	N	77	45	45.029			2300		500	
5	5	Wind Turbine	42	07	15.607	IN N	77	43	13.091			2293		580	
7	7	Wind Turbine	42	07	09.058	IN N	77	42	29.402			2305		580	
/	/	Wind Turbine	42	00	20.110	IN N	77	42	08.845			2344		580	
0	0	Wind Turbine	42	07	56 267	N	77	41	20 227			2205		500	
9 10	9 10	Wind Turbine	42	05	24 124	N	77	43	42 022			2347		586	
10	10	Wind Turbine	42	00	1/ 501	N	77	42	42.033	F	NAD83	2331		586	
12	11	Wind Turbine	42	06	27 587	N	77	42	18 280	F		2275		586	
12	12	Wind Turbine	42	06	27.307	N	77	40	10.200	F	NAD83	2317	NAVD88	586	
14	13	Wind Turbine	42	06	49 190	N	77	40	21 662	F	NAD83	2300	NAVD88	586	
15	14	Wind Turbine	42	05	44 818	N	77	40	05 975	F	NAD83	2302	NAVD88	586	
16	15	Wind Turbine	42	05	06 888	N	77	41	27 580	F	NAD83	2320	NAVD88	586	
17	16	Wind Turbine	42	04	47.401	N	77	42	31.816	E	NAD83	2396	NAVD88	586	
18	17	Wind Turbine	42	04	26.051	N	77	41	18.452	E	NAD83	2274	NAVD88	586	
19	18	Wind Turbine	42	02	58.315	N	77	42	13.499	E	NAD83	2282	NAVD88	586	
20	19	Wind Turbine	42	03	22.547	N	77	41	08.804	E	NAD83	2340	NAVD88	586	
21	20	Wind Turbine	42	02	48.808	N	77	40	22.354	E	NAD83	2279	NAVD88	586	
22	21	Wind Turbine	42	02	13.927	N	77	42	02.887	E	NAD83	2304	NAVD88	586	
23	22	Wind Turbine	42	02	17.585	N	77	41	22.930	E	NAD83	2346	NAVD88	586	
24	23	Wind Turbine	42	02	16.360	Ν	77	39	53.146	Е	NAD83	2344	NAVD88	586	
25	24	Wind Turbine	42	01	58.033	Ν	77	40	37.333	E	NAD83	2279	NAVD88	586	
26	25	Wind Turbine	42	01	06.142	Ν	77	41	18.625	Е	NAD83	2268	NAVD88	586	
27	26	Wind Turbine	42	00	58.241	N	77	40	18.956	E	NAD83	2266	NAVD88	586	
28	27	Wind Turbine	42	00	56.593	N	77	39	48.405	E	NAD83	2269	NAVD88	586	
29	28	Wind Turbine	42	01	27.278	Ν	77	38	26.725	Е	NAD83	2267	NAVD88	586	
30	29	Wind Turbine	42	00	38.083	N	77	41	23.752	E	NAD83	2272	NAVD88	586	
31	30	Wind Turbine	42	00	33.368	Ν	77	39	31.035	Е	NAD83	2238	NAVD88	586	
32	31	Wind Turbine	42	00	54.943	Ν	77	38	18.319	Е	NAD83	2261	NAVD88	586	
Alt 1	Alt 1	Wind Turbine	42	06	14.324	Ν	77	43	05.207	Е	NAD83	2369	NAVD88	586	
Alt 2	Alt 2	Wind Turbine	42	03	46.112	Ν	77	40	01.628	Е	NAD83	2303	NAVD88	586	
Alt 3	Alt 3	Wind Turbine	42	03	30.366	Ν	77	40	14.008	Е	NAD83	2178	NAVD88	586	
Alt 4	Alt 4	Wind Turbine	42	02	28.381	Ν	77	40	26.857	Е	NAD83	2291	NAVD88	586	
SM01	SM01	Met Tower	42	02	08.106	Ν	77	42	16.074	Е	NAD83	2333	NAVD88		

APPENDIX A

	TABLE 2: TRANSMISSION LINE LOCATIONS											
BORING NUMBER	BORING DEPTH (FT.)	PTH RESISTIVITY TEST EASTING NORTHING LONG.		LAT.	LANDOWNER	PARCEL ID	PARCEL #	LAND AGENT VERIFICATION	NOTES			
NOTES:												
LAND ACCES	ND ACCESS SHALL BE COORDINATED AND INALIZED WITH HEXTERA.											
EASTING AN	D NORTHING COO	RDINATES ARE IN N	AD 83 - 3102: NE	W YORK CENTRAL	U.S. SURVEY FT.							
1	25	YES	529012.329	842046.263	-77.65985363	42.30612395	SCHIEDER, JAMES P	535566	EPT4030			
2	25	YES	529/03.5/9	839301.197	-77.65717058	42.29861631	GUILLAUME BLACK FAMILY	535516	EP13980			
3	25	YES	530921.232	837767.000	-77.65259913	42.29444901	BOSSARD, WILLIAM W.	535494	EPT3858			
4	25	YES	530217.487	837063.285	-77.65516713	42.29249395	BOSSARD, WILLIAM W.	535492	EPT3956			
5	25	YES	530075.907	834596.840	-77.65557555	42.28572190	BOSSARD, WILLIAM W.	535504	EPT3968			
6	25	YES	529032.011	832033.594	-77.65931314	42.27865301	SPENCER, J. PHILLIP	535576	EPT4040			
7	25	YES	530243.097	830177.196	-77.65475230	42.27360151	AMIDON FAMILY WEALTH TRUST	535488	EPT3952			
8	25	YES	530184.237	828059.026	-77.65487130	42.26778786	AMIDON FAMILY WEALTH TRUST	535488	EPT3952			
9	25	YES	529420.990	827207.609	-77.65765108	42.26542544	AMIDON FAMILY WEALTH TRUST	535479	EPT3943			
10	25	YES	528322.453	827044.228	-77.66170130	42.26493909	O'LENA, MICHAEL	535575	EPT4039			
11	25	YES	528290.520	825639.414	-77.66175356	42.26108359	PERKINS ORVILLE H III & DONNA	535296	EPT2253			
12	25	YES	528303.394	822996.758	-77.66158247	42.25383339	MURRETT JOHN J & KAREN A	535354	EPT2530			
13	25	YES	528229.153	821572.507	-77.66179008	42.24992308	KOCH DAVID, J	535389	EPT2672			
14	25	YES	528164.016	819596.632	-77.66193826	42.24449960	KOCH DAVID, J	535344	EPT2516			
15	25	YES	528079.339	817028.062	-77.66213088	42.23744925	DINEEN WILLIAM	535165	EPT1831			
16	25	YES	526475.970	817074.795	-77.66805310	42.23752164	SHEARER WILLIAM R & NANCY E TRUST	535270	EPT2192			
17	25	YES	526330.896	814192.601	-77.66845326	42.22960868	HAMMER DARRYL & ROBERT G	535235	EPT2075			
18	25	YES	526237.675	812280.005	-77.66870751	42.22435782	MCCORMICK LINDA DINEEN JAMES	535095	EPT1313			
19	25	YES	526179.583	811058.301	-77.66886453	42.22100378	HAMMER DARRYL & ROBERT G	535235	EPT2075			
20	25	YES	525961.883	806856.905	-77.66947056	42.20946875	CHESAPEAKE ENERGY CORPORATION	535088	EPT1298			
21	25	YES	525883.874	805365.654	-77.66968835	42.20537445	CHESAPEAKE ENERGY CORPORATION	535113	EPT1409			
22	25	YES	525850.167	804493.010	-77.66977171	42.20297898	CHESAPEAKE ENERGY CORPORATION	535113	EPT1409			
23	25	YES	525764.028	802262.935	-77.66998473	42.19685727	CAMPBELL TRUST	535172	EPT1838			
24	25	YES	525711.762	800909.807	-77.67011397	42.19314283	CAMPBELL TRUST	535156	EPT1809			
25	25	YES	523461.623	798344.160	-77.67829468	42.18602446	WILLIAMSON CURT R & ROGER	535247	EPT2123		POTENTIAL ENVIRONMENTAL AREA	
26	25	YES	522227.463	795901.788	-77.68273167	42.17927973	STRUJO JASMIN & SENADA	602381	EPT5340			
27	25	YES	521443.152	794349.655	-77.68555090	42.17499335	BUCK SAMUEL L & SAMUEL C	602477	EPT5436			
28	25	YES	520947.797	793369.359	-77.68733127	42.17228612	WARNER LIVING TRUST	602478	EPT5437			
29	25	YES	520750.160	792978.243	-77.68804156	42.17120599	WARNER LIVING TRUST	602478	EPT5437			
30	25	YES	520712.399	791843.367	-77.68812665	42.16809087	DUNWORTH DEBORAH A	601988	EPT4947			
31	25	YES	520668.585	790526.595	-77.68822537	42.16447646	LYLE TIMOTHY & RICHARD & ROBIN & DAVID	601958	EPT4917			
32	25	YES	520471.827	786312.208	-77.68874971	42.15290636	MCCAFFREY GEORGE M III ETAL	602484	EPT5443			
33	*50*	YES	521748.946	786231.857	-77.68403677	42.15273118	BAR-B-R FARMS INC	602382	EPT5341			
34	25	YES	523010.889	783624.820	-77.67926021	42.14562270	BAR-B-R FARMS INC	602382	EPT5341			
35	25	YES	523282.412	782044.744	-77.67818443	42.14129694	TEIXEIRA JOHN & JACQUELINE	602012	EPT4971			
36	25	YES	522895.161	781471.460	-77.67958495	42.13971037	HAPEMAN ERIC S	602492	EPT5451			
37	25	YES	522863.020	778523.942	-77.67956395	42.13162201	JENKINS THEODORE & MASTROIANNI ANTHONY	602559	EPT5518			
38	*50*	YES	522744.920	775613.072	-77.67986151	42.12363115	KINNER MARY E	602610	EPT5569			
39	25	YES	520178.576	775630.341	-77.68932085	42.12358775	KINNER MARY E	602610	EPT5569			
40	25	YES	520072.084	774064.555	-77.68963860	42.11928786	KINNER MARY E	602610	EPT5569			
41	25	YES	519903.162	771580.838	-77.69014254	42.11246717	SCIENTIFIC POLYMER PRODUCTS INC	602369	EPT5328			
42	25	YES	519196.313	769795.995	-77.69266184	42.10754486	SCIENTIFIC POLYMER PRODUCTS INC	602256	EPT5215			
43	25	YES	518610.603	768317.034	-77.69474909	42.10346609	SCIENTIFIC POLYMER PRODUCTS INC	602256	EPT5215			

1 OF 1

REVISION: E

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	Table	e I - Test B	oring Prog	fram Sumr	nary Eight Points Wil	nd Farm
Component	Total # Borings	Target Depth (feet)	Minimum Depth (feet)	Max. SPT Interval (feet)	Hollow Stem Auger Refusal Criteria	Rock Coring Criteria (feet)
WTG	36	70	40	5	50 blows/inch for first 6 inches and 50 blow/6 inches thereafter	If rock encountered above a depth of 30 feet, core rock until a depth of 40 feet is achieved. If refusal encountered below a depth of 30 feet, no coring necessary
Transmission Line	42	25' except borings #33 and #37 to 50'	15	5	50 blows/inch for first 6 inches and 50 blow/6 inches thereafter	If rock encountered prior to iarget depth, core rock to a minimum depth of 15 feet and a minimum coring length of 5 feet
Substation	6	30	30	5	50 blows/inch for first 6 inches and 50 blow/6 inches thereafter	If rock encountered prior to target depth, core rock until target depth achieved
Collector	0	0	0	0		
O & M Building	1	20	15	5	50 blows/inch for first 6 inches and 50 blow/6 inches thereafter	If rock encountered prior to target depth, obtain 5 foot rock core

TABLE 3 -WI	D TURBINE GEOTECHNICAL SUMMARY									
LOCATION	BEARING STRATUM AT 8' DEPTH	GLACIAL TILL DEPTH BELOW BEARING GRADE								
T-1	HARD GLACIAL TILL	7								
Т-3	HARD GLACIAL TILL	17								
T-4	SANDSTONE	N/A								
T-6	SILTSTONE	N/A								
T-7	SHALE	N/A								
Т-9	WEATHERED ROCK	N/A								
T-10	WEATHERED ROCK	N/A								
T-11	HARD GLACIAL TILL	2								
T-14	WEATHERED ROCK	N/A								
T-16	WEATHERED ROCK	N/A								
T-17	WEATHERED ROCK	N/A								
T-18	HARD GLACIAL TILL	7								
T-21	HARD GLACIAL TILL	9.2								
Т-22	WEATHERED ROCK	N/A								
Т-23	WEATHERED ROCK	N/A								
T-24	HARD GLACIAL TILL	7								
Т-26	HARD GLACIAL TILL	15								
Т-28	HARD GLACIAL TILL	10								
Т-30	WEATHERED ROCK	N/A								
T-31	WEATHERED ROCK	N/A								
ALT-1	WEATHERED ROCK	N/A								
ALT-2	HARD GLACIAL TILL	8.5								

TABLE 4 -TRANSMISSION LINE GEOTECHNICAL SUMMARY											
LOCATION	GLACIOFLUVIAL SOIL THICKNESS (FEET)	GLACIAL TILL THICKNESS (FEET)	DEPTH TO BEDROCK (FEET)								
TL-4	2	8	10								
TL-9	6	19+	UNKNOWN								
TL-10	2.5	22.5+	UNKNOWN								
TL-26	0	25+	UNKNOWN								
TL-29	0	3.5	3.5								
TL-32	2.5	6	8.5								
TL-33	2.5	22.5+	UNKNOWN								
TL-43	6	7.5	13.5								



KEY TO SYMBOLS - GINT STD US LAB.GDT - 9/1/17 15:25 - C:\PROGRAM FILES (X86)\GINT\PROJECTS\2017-054 NEXTERA 8 POINTS.GPJ

KEY TO SYMBOLS



TERMS DESCRIBING CONSISTENCY OR CONDITION

COARSE-GRAINED SOILS (major portions retained on No. 200 sieve): includes (1) clean gravel and sands and (2) silty or clayey gravels and sands. Condition is rated according to relative density as determined by laboratory tests or standard penetration resistance tests.

Descriptive Terms	Relative Density	SPT Blow Count
Very loose	0 to 15 %	< 4
Loose	15 to 35 %	4 to 10
Medium dense	35 to 65 %	10 to 30
Dense	65 to 85 %	30 to 50
Verv dense	85 to 100 %	> 50

FINE-GRAINED SOILS (major portions passing on No. 200 sieve): includes (1) inorganic and organic silts and clays, (2) gravelly, sandy, or silty clays, and (3) clayey silts. Consistency is rated according to shearing strength, as indicated by penetrometer readings, SPT blow count, or unconfined compression tests.

Descriptive Terms	Strength kPa	SPT Blow Count
Very soft	< 25	< 2
Soft	25 to 50	2 to 4
Medium stiff	50 to 100	4 to 8
Stiff	100 to 200	8 to 15
Very stiff	200 to 400	15 to 30
Hard	> 400	> 30

For example; GW-GC, well-graded gravel-sand mixture with clay binder.

Key to Soil Symbols and Terms

GENERAL NOTES

1. Classifications are based on the United Soil Classification System and include consistency, moisture, and color. Field descriptions have been modified to reflect results of laboratory tests where deemed appropriate.

2. Surface elevations are based on topographic maps and estimated locations.

3. Descriptions on these boring logs apply only at the specific boring locations and at the time the borings were made. They are not guaranteed to be representative of subsurface conditions at other locations or times.

			Hard	> 400 > 30										
Ma	ajor Div	risions	Group Symbols	Typical Names			Laboratory Classification	Criteria						
	action size)	gravel no fines)	GW	Well-graded gravels, gravel-sand mixtures, little or no fines			$C_{U} = \frac{D_{60}}{D_{10}}$ greater than 4; $C_{C} = -$	$\frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3		e sizes	200	to #40	to #10	to #4
sieve size)	vels if coarse fr o. 4 sieve	Clean (Little or	GP	Poorly-graded gravels, gravel-sand mixtures, little or no fines	urve, 200	bols**	Not meeting all gradation require	ments for GW	a	Sieve	*	#200	#40 1	#10
Coarse-Grained solis (More than half the material is larger than No. 200 Sands Grave Grained solis (More than half of coarse fraction is smaller than No. 4 sieve size) (More than half of than half of coarse fraction is smaller than No. 4 sieve size) Sands Clean sands Gravel with fines (Appreciable amount of fines)	GM* d	Silty gravels, gravel-sand-silt mixtures	ain size cu	s: g dual sym	Atterberg limits below "A" line or P.I. less than 4	Above "A" line with P.I. between 4 and 7 are border-	icle Size							
	Gravel w (Appre amount	GC	Clayey gravels, gravel-sand-silt mixtures	Wel from gl avel from gl ition smalle id as follow W, SP SM, SC 4s requiring	Atterberg limits above "A" line or P.I. greater than 7	line cases requiring use of dual symbols	Part		_	42	0	.0		
	SW	Well-graded sands, gravelly sands, little or no fines	nd and gra	e classified W, GP, SV GM, GC, 3 Ifine case ²	$C_{U} = \frac{D_{60}}{D_{10}}$ greater than 6; $C_{C} = $.	$\frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3		mm	< 0.074	.074 to 0.4	0.42 to 2.0	2.00 to 4.7		
	lds f coarse fr lo. 4 sieve	Clean (Little or I	SP	Poorly-graded sands, gravelly sands, little or no fines	Iges of sar	ent G ent G rcent	Not meeting all gradation require	ments for SW				0		
	Sar than half o aller than N	ith fines ciable of fines)	SM* d	Silty sands, sand-silt mixtures	e percenta 1g on perce	arse-grain than 5 perc than 12 pe 2 percent	Atterberg limits below "A" line or P.I. less than 4	Above "A" line with P.I. between 4 and 7 are border-		a	clay		m	se
	SC	Clayey sands, sand-clay mixtures	Determin Dependir	Less t Less t More 6 to 1.	Atterberg limits above "A" line or P.I. greater than 7	line cases requiring use of dual symbols	Mate	ואומום	Silt or	Fine	Med	Coar		
i size) (ML Inorganic silts and very fine sand rock floor, silty or clayey fine sand or clayey silts with slight plasticit				80 FOR CL	RIFICATION OF FINE-GRAINED SOIL AND					in.	Ľ	ð in.	
200 sieve	ts and Cla	Liquid limi ss than 60	CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays		70 - FINE-GR	NE-GRAINED FRACTION OF COARSE-GRAINED SOLS					#4 to 3/4 3/4 in. to 3	3 in. to 12	12 in. to 3(
soils ir than No.	ŝ	<u> </u>	OL	Organic silts and organic silty clays of low plasticity	INDEX (PI)									
Fine-Grained soils he material is smaller th and Clays -iquid limit ater than 60)	МН	Inorganic silts, micaceous or disto- maceous fine sandy or silty soils, organic silts		40 – 30 –			Part	E		o 19.1 o 76.2	304.8	o 914.4		
	СН	Inorganic clays of high plasticity, fat clays		20-		MH OR OH		E	* 0L *	4.70 t	76.2 tc	304.8 t		
(More than half th Highly Sitte Organic (L Soils		ОН	Organic clays of medium to high plasticity, organic silts			CL-MLML OR OL 1620 30 40 50 60 7 LIQUID LIMIT (LL)	70 80 90 100 110		<u>a</u>	_	se	e	ers	
		Pt	Peat and other highly organic soils			Plasticity Cha	rt	Mator	ואומרכי	Grave	Coan	Cobb	Boulde	
* D si ** B)ivision o uffix d u Borderlin	of GM ar sed whe e classifi	d SM grou n L.L. is 23	ps into subdivisions of d and u are for roads and a or less and the P.I. is 6 or less; the suffix is used ad for soils possessing characteristics of two grou	irfields o when L.L	nly. Subdivi is greater	sion is based on Atterberg Limits: than 26. combinations of groups symbols.							_



K	Keni	ney	Ge	eotec	hnical Servi	ices							Job No. 2017-054
CI	JENT		8 PO	INTS W	IND, LLC				PROJE	CT EIG	HT POINTS V	WIND ENER(GY PROJECT
EN	IGINEER	ł	SAR	GENT	AND LUNDY, LL	C			LOCAT	TION STE	UBEN COUNT	Y, NY	
	DEPTH	SAMPLE NO.	SAMPLER TYPE Refored	GRAPHIC LOG	BORING NUMBER SURFACE ELEVATION LATITUDE (degrees) 42.062 DESCH (LABOR	ALT-2 LONG 2810 RIPTION OF MAT ATORY CLASSIFI	Sheet ITUDE (degree 77 ERIALS (CATION)	<u>1 of 1</u> 2302.2 es) .667119		Unconfined (1 2 PL PL N 10 20	Compressive Strength	4 5 4 5 1 1 LL 40 50	REMARKS
	- 5.0 — - 10.0 —	1 2 3 4 5			GLACIAL TILL CONSIST HARD SANDY LEAN CL	ING OF RED-BRO AY WITH GRAVE	WN VERY ST L, MOIST	IFF TO					рН 6.08
	- 15.0 —	- 6			HIGHLY WEATHERED S. RUN 1 - 16.5' TO 22.5'	ANDSTONE GRAD	<u>e iv</u> — — —						
ERA 8 POINTS.GPJ	- 20.0 — -	7			RECOVERY = 61.7%. R(FRESH SANDSTONE NATURAL DISCONTINUI RUN 2 - 22.5' TO 27.5'	QD = 15.3% TY WITH CLAY	INFILL AT ~	16.5 FEET					TRANSITIUNAL BEDROCK MAY INCLUDE LARGE DISLODGED ROCK SLABS WITH SOIL INFILLS DUE TO WEATHERING
2017-054 NEX	25.0 —	8			RECOVERY = 40%. RQI FRESH SANDSTONE NATURAL DISCONTINUI) = 15% TY WITH CLAY	INFILL AT ~	23.25 FEET					AVERAGE ROCK CORING RATE = 2 MINUTES PER
36)\GINT\PROJECTS\	30.0 —	9			RUN 3 - 27.5' TO 32.5' RECOVERY = 66%. RQI FRESH SANDSTONE NATURAL DISCONTINUI) = 0% TY WITH CLAY	INFILL AT ~	31.5 FEET					FOOT
OGRAM FILES (X8	35.0 —	10			RUN 4 - 32.5 10 31.5 RECOVERY = 80%. RQI FRESH SANDSTONE	0 = 0%							WATER RETURN AT 35' TO 36'
1/17 14:46 - C:\PKL	40.0 —	11			RUN 5 - 37.5' TO 40.5' RECOVERY = 90%. RQI FRESH SANDSTONE FRESH SHALE End	0 = 6.7% of Boring @ 40.	5 feet	/					LUSS OF WATER RETURN AT 37' TO 38'
403.GDT - 9,	- 45.0				 								
GNGI									•	Calibrated Penetro	meter Unconfined Co	mpression	
00 V					WATER	LEVEL MEASURE	MENTS	1			BORING STAR	TED	6/23/17
03 - L	DAT	TE / 4 T		TI	AE SAMPLEI) C	ASING	CAVE-1	IN	WATER	BORING COMP	LETED	6/23/17
GNGN	6/23 6/23	/17		10: 10-	UU BCR 30 ACR		-	75		38.8 DRV	DRILLER	PW	RIG CME 550
00 0	J/ 20	/ 11	+	10.				1.5		D RI	DRAWN BY	D ALLEN	APPROVED
								1			11	ע החחרוא	





Ken	ney	Ge	eote	chnic	al Services	5								J	job No. 2017-054
CLIENT		8 PO	INTS V	VIND, LLO	C			PROJE	CT]	EIGHT	POINT	S WINI) ENER(GY PR	OJECT
ENGINEE	R	SAF	GENT	AND L	UNDY, LLC			LOCAT	ION	STEUB	EN COU	NTY, N	YY		
DEPTH	SAMPLE NO.	SAMPLER TYPE	GRAPHIC LOG	BORING D	NUMBER E ELEVATION DE (degrees) 42.114696 DESCRIPTION (I RODETTORY	T-4 Sheet	<u>1 of 1</u> 2360.0 ²⁵⁾ .729176		Unconfin	ned Comp 2 I N VA	ressive Stre: 3 H MC LUE, blows/ - \bigcirc	ngth, tons/	/ft. ² 5 ↓ ▲		REMARKS
	1	V		BROWN	STIFF SANDY SILT WI	TH GRAVEL, MOIST				20	30	40	50		
5.0 -	- 2 - 3 - 4 - 5			HIGHLY	WEATHERED SANDSTO	NE GRADE IV									0 pH 8.08 0
10.0 -	- 6			RUN 1 RECOVE FRESH	<u>- 10.0' TO 15.0'</u> RY = 100%. RQD = 10 SANDSTONE									· — —-	TRANSITIONAL BEDROCK MAY INCLUDE LARGE DISLODGED ROCK
15.0 -	- 7	-		RUN 2 RECOVE FRESH	- 15.0' TO 20.0' RY = 100%. RQD = 6. SANDSTONE	7%									SLABS WITH SOIL INFILLS DUE TO WEATHERING
	- 8			RUN 3 RECOVE FRESH	- 20.0' TO 25.0' RY = 94%. RQD = 50% SANDSTONE	∕₀									AVERAGE ROCK CORING RATE = 2-3 MINUTES PER FOOT
- 0.25 - 0.25 - 0.25	- 9			RUN 4 RECOVE FRESH	- 25.0' TO 30.0' RY = 84%. RQD = 0% SANDSTONE	6									
30.0 -	- 10			RUN 5 RECOVE FRESH	- 30.0' TO 35.0' :RY = 66%. RQD = 10 SHALE	1%									
35.0 -	35.0 - - 11 RUN 6 - 35.0' TO 40.0' RECOVERY = 86% RQD = 8% FRESH SHALE														
40.0 -	-				End of Bori	ng @ 40 feet				<u> </u>			[
45.0 -			L					│ ₩	Calibrated Pe	netromete	r Unconfine	 1 Compress	 ion		
					WATER LEVEL 1	MEASUREMENTS		<u> </u>			BORING S	TARTED		7/10	/17
DA	TE		Т	IME	SAMPLED	CASING	CAVE-	IN	WATER	1	BORING C	OMPLETED		7/19	/ 11
7/19 7/19 7/19	9/17 9/17		16 17	:30 :30	BCR ACR	10 -	- 28.1	1	DRY DRY	, ,	DRILLER DRAWN_B	Y	PW	RIG APPRO	CME 550 VED
2											N	. MORI	HOUSE		



-OG A GNGN03 - LOG A GNGN03.GDT - 9/1/17 14:46 - C:/PROGRAM FILES (X86)/GINT/PROJECTS/2017-054 NEXTERA 8 POINTS.GP.

Ken	ney	Ge	eotec	hnic	al Services	5							J	ob No. 2017-054
CLIENT	-	8 PO	INTS W	IND, LLC	C			PROJI	СТ	EIGHT	POINTS	WIND ENE	RGY PR	OJECT
ENGINEEF	8	SAF	GENT	AND L	UNDY, LLC			LOCA	TION	STEUE	BEN COUN	ITY, NY		
				BORING 1	NUMBER				Uncor	nfined Comp	pressive Streng	th, tons/ft. ²		
		벖	5	SURFAC	E ELEVATION	T-6 Sheet	1 of 1		1	2	 3	4 5		
EPTH	PLE NO	LER TY	HIC TO	1.8919111	DF (dogroop)	I ONCITUDE (Jourse	2365.0		זמ		MC			MARKS
	SAM	SAMP	GRAF	LAIIIU	42.119183		. 708184		\times	N VA				RE
					DESCRIPTION (LABORATORY	I OF MATERIALS CLASSIFICATION)			10	00				
	1	Χ		BROWN	STIFF GRAVELLY SILT	WITH SAND, MOIST		<			30	40 50		
-	-			HIGHLY	WEATHERED SHALE G	RADE IV	7							TRANSITIONAL
5.0 —	2			RECOVE FRESH	- 2.5 10 1.5 RY = 90%. RQD = 0% SHALE	Ó							1	BEDROCK MAY INCLUDE LARGE DISLODGED ROCK SLABS WITH
-	-			RUN 2 RECOVE	- 7.5' TO 12.5' RY = 88%. RQD = 0%	, 0								SOIL INFILLS DUE TO WEATHERING
10.0 -	3			LKF2H	PHAFF									
15.0	4			RUN 3 RECOVE FRESH	-12.5' TO 17.5' RY = 100%. RQD = 1; SANDSTONE	3%								CORING RATE = 2-3 MINUTES PER FOOT
₩D - 20.0 -	- 5			RUN 4 RECOVE FRESH	- 17.5' TO 22.5' 'RY = 48%. RQD = 0% SANDSTONE	Ó								
- 25.0 -	6			RUN 5 RECOVE FRESH	- 22.5' TO 27.5' RY = 90%. RQD = 449 SHALE	%								
₩₩ - 30.0	7			RUN 6 RECOVE FRESH	- 27.5' 32.5' RY = 88%. RQD = 09 SHALE	%								
35.0 -	8		× × × × × × × × × × × × × × × ×	RUN 7 RECOVE FRESH	- 32.5' TO 37.5' RY = 100%. RQD = 4 SILTSTONE	5%								
40.0	- 9			RUN 8 RECOVE	- 37' TO' RY = 87%. RQD = 0% SILTSTONE End of Borir	6 ng @ 40.5 feet								
45.0]					Calibrated	 Penetrometo	er Unconfined			
					WATER LEVEL	MEASUREMENTS					BORING STR	IRTED	7/10	/17
DAT	re 		TII	Æ	SAMPLED	CASING	CAVE-	IN	WAT	TER	BORING CON	APLETED	7/18	/17
7/18	/17	+	09: 09:	30	WD BCR	20.0	-		18 28	.0	DRILLER	P	RIG RIG	CME 550
7/18	/17		10:	00	ACR	-	23		28	.0	DRAWN BY	D. ALLE	APPRO'	VED

Ken	ney	G	eote	chnic	al Services	5								J	ob No. 2017-054
CLIENT		8 P(DINTS	WIND, LLO	C			PROJE	CT	EIGHT	POINTS	WIND	ENERGY	Y PR	OJECT
ENGINEER	R	SAI	RGENT	r and l	UNDY, LLC			LOCA	TION	STEUE	BEN COU	NTY, NY			
	NO.	TYPE	200	BORING SURFAC	NUMBER E ELEVATION	T-7 Sheet	1 of 1		Unconf	ined Comp	ressive Streng	gth, tons/ft. ² 4	5		53
DEPTH	SAMPLE	SAMPLER	KECUVEJ GRAPHIC	LATITU	DE (degrees) 42.115586 DESCRIPTION	LONGITUDE (degree 77.	2344.0 ⁽⁵⁾ .702457		PL X	N VA	MC LUE, blows/f	ц Ц т.	I		REMAR
				BROWN	(LABORATORY STIFF GRAVELLY SILT	CLASSIFICATION) WITH SAND, MOIST, (RGANICS		10	20		40	50		
-	2			GLACIA WITH G	L TILL CONSISTING OF RAVEL, MOIST	BROWN HARD CLAYEY	' SAND								
- 5.0	4			GLACIA SAND V HIGHLY	L TILL CONSISTING OF VITH GRAVEL, MOIST WEATHERED SHALE G	RED-BROWN HARD CL	AYEY							 64 70	
10.0	- -			RUN 1	- 12.0' TO 17.0'										TRANSITIONAL
wp 15.0 - ⊻	6			RECOVE FRESH	RY = 100%. RQD = 78 SANDSTONE	3%]	BEDROCK MAY INCLUDE LARGE DISLODGED ROCK SLABS WITH SOIL INFILLS DUF TO
- 20.0	7	Ι		RUN 2 RECOVE FRESH	- 11.0 10 22.0 RY = 94%. RQD = 34% SANDSTONE SHALE	/0									WEATHERING
- 25.0 -	8		*****	RUN 3 RECOVE	- 22.0' TO 27.0' RY = 96%. RQD = 149 SILTSTONE	/6		_							AVERAGE ROCK CORING RATE = 3-4 MINUTES PER FOOT
₩CR - - 	9		× × × × × × × × × × × × × × × × × × ×	RUN 4 RECOVE	- 27.0' TO 32.0' RY = 100%. RQD = 41 SILTSTONE	1.7%									
35.0 -	10			RUN 5 RECOVE	- 32.0' TO 37.0' RY = 90%. RQD = 0% SILTSTONE										
40.0	11			RUN 6 RECOVE	- 32.0' TO 40.0' :RY = 100 %. RQD = 2 SILTSTONE Find of Pori	29%									
-	-				Liiu vi boli	иу (£ 10 16£1									
45.0 -	<u> </u>		· 上 _ ·					•	Calibrated P	enetromete	er Unconfined	Compression			
	TP			IMP	WATER LEVEL I	MEASUREMENTS		TN			BORING ST	ARTED	1	1/17/	/17
7/17	1E 2/17	-	T 09	1ME):00	SAMPLED	CASING 17.0	CAVE-	IN	WATE 16.(к)	BORING CO	MPLETED		1/17	/17
7/17	/17		0	9:30	BCR	32.0	-		28.0)	DRILLER		PW	RIG	CME 550
7/17	/17		1():00	ACR	-	21.7	7	28.0)	DYYANN RA	D. A	LLEN	APPKU	עדא

Kenr	ney	Ge	eotec	hnical Services								Job No. 2017-05
CLIENT		8 PO	INTS W	IND, LLC			PROJEC	T E	EIGHT POIN	ITS WIND E	NERGY P	ROJECT
INGINEER	1	SAR	RGENT	AND LUNDY, LLC			LOCATI	ON S	TEUBEN C	DUNTY, NY		
	VO.	YPE	100	BORING NUMBER SURFACE ELEVATION	T-9 Sheet	1 of 1		Unconfin	ed Compressive S	trength, tons/ft. ²	5	2
DEPTH	SAMPLE 1	SAMPLER 1	GRAPHIC]	LATITUDE (degrees) 42.098992	LONGITUDE (degree 77	2346.7 es) 7.722288		PL ×			I	REMARK
				DESCRIPTION (LABORATORY)	OF MATERIALS CLASSIFICATION)			10 9		40 40		
	1	X	° `° ()	BROWN LOOSE GRAVELLY SANI	O WITH SILT, MOIST		\Diamond					рН 6.22
5.0 —	2	X		GLACIAL TILL CONSISTING OF CLAY WITH GRAVEL, MOIST	RED-BROWN HARD SA	INDY LEAN						
10.0 —	3	X		HIGHLY WEATHERED SANDSTO	NE GRADE IV						10	00
- 15.0 —	5			RUN 1 - 12.1' TO 17.1' RECOVERY = 50%. RQD = 4.2% FRESH SANDSTONE								TRANSITIONAL BEDROCK MAY INCLUDE LARGE DISLODGED ROCK SLABS WITH SOIL INFILLS
- 20.0 —	6			RUN 2 - 17.1' TO 22.1' RECOVERY = 28%. RQD = 0% FRESH SANDSTONE								DUE TO WEATHERING
- 25.0 —	7			RUN 3 - 22.1' TO 27.1' RECOVERY = 48%. RQD = 0% FRESH SANDSTONE NATURAL DISCONTINUITY WIT	h clay infill at ~	26.5 FEET						WATER USED WHILE ROCK CORING
- 30.0 —	8			RUN 4 - 27.1' TO 32.1' RECOVERY = 100%. RQD = 42. FRESH SANDSTONE NATURAL DISCONTINUITY WIT	5% H CLAY INFILL AT ~	29.2 FEET						AVERAGE ROCK
- 35.0 —	9			RUN 5 - 32.1' TO 37.1' RECOVERY = 100%. RQD = 44. FRESH SANDSTONE	2%	29 5 EEEm						4.1 MINUTES PER FOOT
- 40.0 —	10			RUN 6 - 37.1' TO 42.1' RECOVERY = 96%. RQD = 61.6 FRESH SANDSTONE FRESH CONGLOMERATE FRESH SANDSTONE	и онат INfibb AI ~ %							
45.0 —				End of Boring	g @ 42.1 feet							
				WATER LEVEL M	IEASUREMENTS		•		BORIN	G STARTED	A /A	/47
DAT	ĨE		TI	ME SAMPLED	CASING	CAVE-	IN	WATER	BORIN	G COMPLETED	6/6/	/17
6/6/	6/6/17			30 BCR	12.1 BECODDE	- D DEFAD	E DUUM	DRY	DRILLE	R	6/6/ RGES	7899
6/6/	/ 11	+	13:	JU BCK	RECORDE	N RELOK	F KOCK	CURING	DRAWI	N BY	APPR	0VED



Ken	iney	G	ec	otec	hnica	al Services	5								Job No. 2017-054
CLIENT		8 P	OIN	TS W	IND, LLC	;			PROJE	CT	EIGHT	POINT	S WIND I	ENERGY	PROJECT
ENGINE	ER	SA	RG	ENT	AND L	UNDY, LLC			LOCAT	ION	STEUB	EN COU	NTY, NY		
HLd3Q	SAMPLE NO.	SAMPLER TYPE	RECOVERY	GRAPHIC LOG	BORING N SURFACI LATITUL	IUMBER E ELEVATION DE (degrees) 42.107663 DESCRIPTION	T-11 Sheet LONGITUDE (degree -77, OF MATERIALS	1 of 1 2317.0 ²⁵⁾ .768841		Uncon	fined Comp	Image: stressive stress	ngth, tons/ft. ² 4 LL £	5	REMARKS
	1		-		BROWN	LABORATORY LOOSE SANDY SILT W	TTH GRAVEL AND ORG	ANICS,		10	20	30	40	50	
5.0					MOIST GLACIAI WITH S	I TILL CONSISTING OF AND AND COBBLES, MO	BROWN HARD GRAVEL OIST	TA CTUA							
10.0	- 5				HIGHLY	WEATHERED SHALE G	RADE IV, MOIST								
15.0	6														100 100
20.0	- 8				RUN 1 - RECOVE FRESH S NATURA	20.0' TO 25.0' RY = 80%. RQD = 38. SANDSTONE L DICONTINUITY AT 2	3%								TRANSITIONAL BEDROCK MAY INCLUDE LARGE DISLODGED ROCK SLABS WITH
0.62 NIECTS/2017-054 NI	- 9				RUN 2 - RECOVE FRESH S	25.0' TO 30.0' RY = 84%. RQD = 309 SANDSTONE	%								SOLI INFILLS DUE TO WEATHERING
30.0 X86)/GINT/PRC	- 10				RUN 3 - RECOVE FRESH S	30.0' TO 35.0' RY = 40%. RQD = 0% SANDSTONE	5								AVERAGE ROCK CORING RATE = 3-4 MINUTES PER FOOT
35.0	- 11				RUN 4 - RECOVE FRESH S	- 30.0' TO 40.0' RY = 40%. RQD = 0% SHALE									
17 - C:\F					NATURA	L DISCONTINUITY AT	36' WITH CLAY INSEA	М							
45.0 45.0											r Unconfine	d Compression			
06 4 6						WATER LEVEL	MEASUREMENTS					BORING S	TARTED	7	/12/17
M - C	ATE	-		TIN	E	SAMPLED	CASING	CAVE-	IN	WAT	ER	BORING C	OMPLETED	7	/12/17
7/1 7/1	7/12/17 7/12/17			15: 16:	30 30	BCR ACR	<u>40</u> -	31.2	2	26 26	5 5	DRILLER DRAWN B	Y Modeu	PW F	RIG CME 550 IPPROVED
45.0 45.0 7/1: 7/1: 7/1:	45.0 Image: Second state					WATER LEVEL SAMPLED BCR ACR	MEASUREMENTS CASING 40 -	CAVE-1 - 31.2	IN 2	Calibrated WAT	Penetromete ER 5	r Unconfine BORING S BORING C DRILLER DRAWN B N	d Compression TARTED OMPLETED Y . MOREH		

Ker	ne	y G	e	otec	hnical Services	5								Job No. 2017-054
CLIENT		8 P	0 I	NTS W	IND, LLC			PROJEC	T I	EIGHT	POINT	'S WIND E	NERGY P	ROJECT
ENGINE	ER	SA	RQ	GENT	AND LUNDY, LLC			LOCATI	ION S	STEUB	EN COI	J NTY, NY		
					BORING NUMBER	T-14 Sheet	1 of 1		Unconfin	ned Comp	ressive Stre - ()	ength, tons/ft. ²		
EPTH	PLE NO.	LER TYPE	OVERY	HIC LOG	SURFACE ELEVATION	I ONCIPIINE (Jose	2327.5			2	3 MC	4	5	MARKS
	SAM	SAMP	RE(GRAF	42.095783	77	.684993		×—	N VAI	UE, blows	/ft.		RE
				• • • • • •	LABORATORY (LABORATORY	CLASSIFICATION)	чт. шо		10 2	20	-\$	40	50	
	1				HARD SANDY SILT WITH GRA	VEL, MOIST	11 10	\diamond						DRILLER NOTES
5.0	- 3							C				*		POSSIBLE COBBLES AND BOULDERS
	- 4			<u>), , , , , , , , , , , , , , , , , , , </u>	HIGHLY WEATHERED SANDST	ONE GRADE IV						\rightarrow		-
10.0	5												<	TRANSITIONAL BEDROCK MAY
15.0	6				RUN 1 - 12' TO 17' RECOVERY = 82%. RQD = 20 FRESH SANDSTONE									- INCLUDE LARGE DISLODGED ROCK SLABS WITH SOIL INFILLS DUE TO WEATHERING
	7				RUN 2 - 17' TO 22' RECOVERY = 90%, RQD = 35 FRESH SANDSTONE	.8%								LOSS OF WATER RETURN AT 15.5'
25.0 25.0	- 8				RUN 3 - 22' TO 27' RECOVERY = 100%. RQD = 5 FRESH SANDSTONE NATURAL DISCONTINUITY WI AND ~23.5 FEET	8.3% Th clay infill at ~	22.4 FEET							575 GALLONS OF WATER USED WHILE ROCK CORING
30.0 30.0	9				RUN 4 - 27' TO 32' RECOVERY = 96%. RQD = 39 FRESH SANDSTONE NATURAL DISCONTINUITY WI	.2% TH CLAY INFILL AT ~	29.5 FEET							AVERAGE ROCK
XAM FILES (X86)/GI 35.0	10				RUN 5 - 32' TO 37' RECOVERY = 96%, RQD = 58 FRESH SANDSTONE	.3%								3.5 MINUTES PER FOOT
0.04 C:/PROG	- 11				RUN 6 - 32.2' TO 37.2' RECOVERY = 100%. RQD = 6 FRESH SANDSTONE FRESH CONGLOMERATE	3.9%	7							-
1/1/1/1					End of Bor	ıng @ 40 feet								
5.0 45.0	<u> </u>			L				•	 Calibrated Per	netromete	r Unconfine	 ed Compression		\bot
		1			WATER LEVEL	MEASUREMENTS		I			BORING S	STARTED	6/5	/17
	ATE 5/17			TIN 15-	IE SAMPLED 00 BCR	CASING 40	CAVE-	IN	WATER 19_3	2	BORING (COMPLETED	6/5/	/17
6/9 6/9	5/17			15:	00 ACR	-	11.4	4	DRY		DRILLER DRAWN I	ן איז מי	KGES RIG	7822DT
Ĭ												J . A	utitii	

Keni	ney	G	e	otec	hnic	al Services	5								Job No. 2017-054
CLIENT		8 P	110	NTS W	'IND, LLC	;			PROJEC	T E	EIGHT	POINT	S WIND	ENERGY	PROJECT
ENGINEEF	R	SA	RG	ENT	AND L	UNDY, LLC			LOCATI	ION S	TEUE	BEN CO	UNTY, NY	Y	
		PE		5	BORING N SURFAC	NUMBER , E ELEVATION	T-16 Sheet	1 of 1		Unconfin	led Com <u>r</u> 2	oressive Str — () — 3	ength, tons/ft 4	5	-
DEPTH	SAMPLE NO	SAMPLER TYI	RECOVERY	GRAPHIC LO	LATITUI	DE (degrees) 42.079834	LONGITUDE (degree 77.	2396.0 ²⁵⁾ .708838		PL ×	N 1/1	MC		1	REMARKS
						DESCRIPTION (LABORATORY	OF MATERIALS CLASSIFICATION)			10 2	N V #	1101, diows 	/ n. 40	50	
	1	X			GLACIAI WITH G	L TILL CONSISTING OF RAVEL, MOIST	BROWN HARD SANDY	SILT		\diamond					
- 5.0	2	X			MODERA	ATELY WEATHERED SA	NDSTONE GRADE III								 100
10.0	5	-			RUN 1 RECOVE FRESH 2 RUN 2 RECOVE FRESH 2	- 5.0' TU 6.0' RY = 0.0%. RQD = 0 SANDSTONE - 6.0' TO 11.0' RY = 68.0%. RQD = 0 SANDSTONE	%								TRANSITIONAL BEDROCK MAY INCLUDE LARGE DISLODGED ROCK SLABS WITH SOIL INFILLS DUE TO
	6 15.0 - RUN 3 11.0' TO 16.0' RECOVERY = 100% RQD = 0% FRESH SANDSTONE NATURAL DISCONTINUITY AT ~10.4 FEET														WEATHERING AVERAGE ROCK CORING RATE = 2.5 MINUTES
	7				RUN 4 - RECOVE FRESH S	- 16.0' TO 21.0' RY = 70%. RQD = 09 SHALE	%								PER FOOT
20.0	-				HIGHLY RUN 5 -	WEATHERED SHALE G - 21.0' TO 26.0' BY = 80% RDD = 25'	RADE IV								AVERAGE ROCK CORING RATE = 0.5 MINUTES PER FOOT
BCR ▼ 25.0 -	8				FRESH S	AL DISCONTINUITY AT	~23.5 FEET AND ~24.	9 FEET							
-	9				RUN 6 - RECOVE FRESH S	- 26.0' TO 31.0' RY = 74%. RQD = 8.3 SHALE	%								WATER RETURN AT 26' LOSS OF WATER RETURN AT 27.5'
30.0	10				RUN 7 - RECOVE	- 31.0' TO 35.0' RY = 130%. RQD = 49 SANDSTONE	9%								ROCK SAMPLE WAS RETRIEVED FROM PREVIOUS
35.0	$35.0 - \begin{bmatrix} 10 \\ - \\ - \\ - \\ - \\ - \\ 11 \end{bmatrix}$ RUN 8 - 21.0' TO 26.0' RECOVERY = 62%. RQD = 13.3% FRESH SANDSTONE														RUN
40.0	-					End of Borir	ıg @ 40.5 feet		_						_
						WATER LEVEL	MEASUREMENTS		U			BORING	STARTED	-	
DAT	FE			TII	VIE	SAMPLED	CASING	CAVE-	IN	WATER		BORING	COMPLETED	6/	/30/17
7/5	7/5/17				:00	BCR	40.5	-		25.1		חפון ודס		7/	5/17
7/5	/17			10:	30	ACR	-	5		DRY		VIIIII AU	DV	PW	CME 550
												DKAWN	D. I	ALLEN	LLVOATD

CLIENT														,
THATWAT		8 P 0	INTS	WIND, LL	C			PROJECT	r]	EIGHT	POINTS	WIND ENH	ERGY	PROJECT
ENGINEER	R	SAR	GEN	r and l	UNDY, LLC			LOCATIO	DN	STEUB	EN COUI	NTY, NY		
				BORING	NUMBER	Г-17 Sheet	1 of 1		Unconfi	ned Compr	essive Streng	gth, tons/ft.²		-
PTH .	PLE NO.	LR TYPE	HIC LOG	SURFAC	CE ELEVATION		2273.8		1	2	3	4 5		IARKS
IQ	SAMI	SAMPI	GRAP	LATITU	DE (degrees) 42.073903	LONGITUDE (degree	es) .688459		<u>к</u>	N VAI.	MC B IIE blows/f	يليا ک		REN
					DESCRIPTION (LABORATORY	OF MATERIALS CLASSIFICATION)			10	20	- <>	40 50)	
	1	X		MODER	ATELY WEATHERED SA	NDSTONE GRADE III						\diamond		_
-	2	Χ												100
5.0														
-	- 3			RUN 1 RECOVE FRESH	- 5.5' TO 9.5' ERY = 100%. RQD = 09 SANDSTONE									TRANSITIONAL BEDROCK MAY INCLUDE LARGE DISLODGED ROCK
10.0 -	-			RUN 2 RECOVE	AL DISCONTINUITY WIT - 9.5' TO 14.5' ERY = 68%. RQD = 0% SINDSTANT	TH CLAY INFILL AT ~	8.6 FEET							SLABS WITH SOIL INFILLS DUE TO WEATHERING
15.0	4			E BIIN 5	- 145' TO 190'									
- 15.0	5			RECOVE	ERY = 98%. RQD = 169 SANDSTONE	~ 18 1 FFFT								AVERAGE ROCK CORING RATE = 2 MINUTES PER FOOT
20.0 —	6			RUN 4 RECOVE	- 19.0' TO 24.0' ERY = 100%. RQD = 7. SANDSTONE	5%								
25.0 —	7	-		RUN 5 RECOVE FRESH	- 24.0' TO 29.0' ERY = 100%. RQD = 33 SANDSTONE	3.3%								WATER RETURN AT 25'
-				NATUR	AL DISCONTINUITY \sim 2	5 FEET AND ~27 FEET	ſ							LOSS OF WATER RETURN AT 25.5'
30.0	8			RUN 6 RECOVE FRESH	- 29.0' TO 33.5' ERY = 89%. RQD = 749 SANDSTONE	Ve								
BCR 35.0 —	9			RUN 7 RECOVE FRESH	- 33.5' TO 38.2' ERY = 106%. RQD = 80 SANDSTONE	3%								ROCK SAMPLE WAS RETRIEVED FROM PREVIOUS RUN
40.0	10			RUN 8 RECOVE	- 38.2' TO 40.2' ERY = 90%. RQD = 50% SANDSTONE									WATER RETURN AT 38' LOSS OF WATER RETHIRN AT 39'
-					End of Born	ıg @ 40.2 teet								
					WATER LEVEL	MEASUREMENTS					BORING ST	ARTED	6/	28/17
DAT	FE		1	IME	SAMPLED	CASING	CAVE-	IN	WATER	R	BORING CO	MPLETED	6/	<u> </u>
6/29	/17	+	1):00)-45	BCR	38.2	-		35.8 עפת	6	DRILLER	P	w RI	G CME 550
0/ 43									DUI	·	DRAWN BY	D_ AI.I.I	EN AF	PPROVED

K	enr	ıey	G	e	ote	chnic	al Services	5								Job No. 2017-054
CLI	ENT		8 P	01	NTS V	WIND, LL	C			PROJI	СТ	EIGHT	POINT	S WIND EN	VERGY PH	ROJECT
ENG	GINEER		SA	RC	ENT	TAND I	LUNDY, LLC			LOCA	FION	STEUE	BEN COU	NTY, NY		
						BORING	NUMBER	T-18 Sheet	1 of 1		Unco	nfined Comp	ressive Stren	19th, tons/ft.²		
пшца	ПТЧ	PLE NO.	LER TYPE	OVERY	HIC LOG	SURFA	CE ELEVATION	I ONCITIUNT (JOSTO	2282.0		1 	2	3 	4	5	MARKS
	-	SAM	SAMP	REC	GRAF		42.049532		.037496		×	N VA	LUE, blows/:	ft.		KE
				-		BROW	DESCRIPTION (LABORATORY N STIFF SANDY SILT W	CLASSIFICATION)			10	20		40	50	
	-	1 2				GLACIA CLAY	AL TILL CONSISITNG OF WITH GRAVEL, MOIST	RED-BROWN HARD SA	NDY LEAN						10	DRILLER NOTES POSSIBLE
	5.0 —	3									ו					COBBLES AND BOULDERS pH 6.98
	-	4	X											>		
	10.0 —	5												∽		
	-															
	15.0 —				HD.	RUN 1 RECOV	- 15.0' TO 20.0' ERY = 74%. RQD = 18	3%								TRANSITIONAL BEDROCK MAY
GPJ	_	6	X		· · · · ·	FRESH NATUR	SANDSTONE RAL DISCONTINUITY AT	16.8'								INCLUDE LARGE DISLODGED ROCK SLABS WITH
S POINTS.	20.0 -				· · · · ·	RUN 2 RECOV	- 20.0' TO 25.0' ERY = 100%. RQD = 4	13.3%								SOIL INFILLS DUE TO WEATHERING
EXTERA 8	-	7	Å			FRESH NATUR	SANDSTONE RAL DISCONTINUITY AT	20.0'								AVERAGE ROCK CORING RATE =
017-054 N	25.0 —			-		FRESH	CONGLOMERATE		7							3-4 MINUTES PER FOOT
OJECTS/2	_	8	Å			RECOV FRESH	- 25.0 10 50.0 ERY = 50%. RQD = 8.3 SANDSTONE	%								
)/GINT/PR	30.0 —		V			RUN 4 RECOV	AL DISCONTINUITY AT - 30.0' TO 35.0' ERY = 50%. ROD = 10	26.2' 0%								
ILES (X86	25.0	9				FRESH	SANDSTONE									
OGRAM F	JO.U	10	V			RUN 5 RECOV FRESH	- 35.0' TO 40.0' ERY 58%. RQD = 44.2% SANDSTONE									
:48 - C:\PF	40.0 Ford of Paring (2) 40 foot															
- 9/1/17 14	-						End of Born	шу (Ш 4V IEEI								
N03.GDT	45.0 -															
A GNG							WATER LEVEL	MEASUREMENTS			rainigiag	renettomet	BORING ST	TARTED		//=
- LOG	DAT	E			Т	IME	SAMPLED	CASING	CAVE-	IN	WA	TER	BORING CO	OMPLETED	7/6/	<u>'17</u>
NGN03	7/6/17 15				15	:00	BCR	40	-		22	.5	DRILLER		7/6/ RIG	<u>/ 17</u>
DG A G	7/6/17 15:30 ACR -							13		DF	RY	DRAWN_B	MODELLA	PW APPR	OVED	
2													N.	. MUKEHO	02E	
Ker	iney	G	eote	chnic	al Services	5							Job No. 2017-054			
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CLIENT		8 PC	DINTS	WIND, LL	C		PROJE	EIGHT POINTS WIND ENERGY PROJECT								
ENGINE	ER	SAI	RGEN	r and l	UNDY, LLC		LOCAT	ATION STEUBEN COUNTY, NY								
	0.	r PE	. 90	BORING SURFAC	NUMBER ,	Г-21 Sheet	1 of 1		Unconfi 1	ned Comp	oressive Stren	gth, tons/ft. ²	5	-		
DEPTH	SAMPLE N	SAMPLER TY	KECUVER) GRAPHIC L	LATITU	LATITUDE (degrees) 42.037202	230 LONGITUDE (degrees) 77.700	2303.7 es) .700802		PL ×	I N VA	MC LL ALUE, blows/ft.		I	REMARKS		
					(LABORATORY	CLASSIFICATION)			10	20	-\$ 30	40				
	1	X		BROWN	LOOSE GRAVELLY SAN	D WITH SILT, MOIST		\diamond								
5.0	2			GLACIA CLAY V	L TILL CONSISTING OF VITH GRAVEL, MOIST	RED-BROWN HARD SA	NDY LEAN					×				
10.0	4	X												100briller notes POSSIBLE COBBLES AND BOULDERS		
15.0	5	X									\diamond			100		
8 POINTS.GPJ	_ 7			RUN 1 RECOVI FRESH	- 17.2' TO 22.2' ERY = 86%. RQD = 559 SANDSTONE.	Va								TRANSITIONAL BEDROCK MAY INCLUDE LARGE DISLODGED ROCK SLABS WITH SOIL INFILLS		
217-054 NEXTERA	8			RUN 2 RECOVI FRESH	- 22.2' TO 27.2' ERY = 100%. RQD = 80 SANDSTONE.]%								DUE TO WEATHERING LOSS OF WATER RETURN AT 21.0'		
0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	9		• • • • • •	RUN 3 RECOVI FRESH	- 27.2' TO 32.2' CRY = 100%. RQD = 70 SANDSTONE.]%								600 GALLONS OF WATER USED WHILE ROCK CORING		
35.0 35.0 35.0	10			RUN 4 RECOVI FRESH	- 32.2' TO 37.2' ERY = 100%. RQD = 7(SANDSTONE. CONGLOMERATE	6.6%								AVERAGE ROCK CORING RATE =		
14:48 - C:\PROC 006	11			RUN 5 RECOVI FRESH FRESH	- 37.2' TO 40.2' ERY = 100%. RQD = 69 CONGLOMERATE SANDSTONE	9%	7							3.4 MINUTES PER FOOT		
03.GD1 - 9/1/17					FUG OF ROLL	ıy (W 4V.Z IEEt										
45.0 NON D							•	Calibrated Pe	netromete	er Unconfined	Compression					
00 V				17.67	WATER LEVEL	MEASUREMENTS					BORING ST	ARTED	6/	/1/17		
1	DATE 6/1/17		1 1	IME B:30	SAMPLED	CASING 27_2	CAVE-	IN	WATER 23 5	x	BORING COMPLETED			6/1/17		
6/1	6/1/17			3:30	ACR		15	ZJ.J DRY		<u> </u>	DRILLER KGES		KGES	RIG 7822DT		
00											DRAWN BY	D. A	LLEN	LLKOAFD		



.06 A GNGN03 - LOG A GNGN03.GDT - 9/1/17 14:48 - C:\PROGRAM FILES (X86)\GINT\PROJECTS\2017-054 NEXTERA 8 POINTS.GP

Ke	Kenney Geotechnical Services Job No. 2017													Job No. 2017-054		
CLIEN	CLIENT 8 POINTS WIND, LLC										PROJECT EIGHT POINTS WIND ENERGY PROJECT					
ENGIN	ENGINEER SARGENT AND LUNDY, LLC										STEUE	BEN COU	NTY, NY			
					BORING	NUMBER				Unconfi	ned Comp	ressive Stre	ngth, tons/ft. ²			
			ĘЧ		SURFAC	CE ELEVATION	<u>Г-23 Sheet</u>	1 of 1		1	2	-0— 3	4	5		
HLd	ON LI	ЛИ ЛП.	ER TYP	IIC TOC				2343.6			1			1	ARKS	
DE	C R ML	DAME	SAMPL	GRAPI	LATITU	DE (degrees) 42 037878	LONGITUDE (degre	jrees) 77.664762		PL X					REW	
						DESCRIPTION	OF MATERIALS	1001102			N VA	LUE, blows/ —�	ft.			
				971))	GLACIA	(LABORATORY L TILL CONSISTING OF	CLASSIFICATION) RED-BROWN VERY ST	IFF TO	1	0	20	30	40	50	BORING OFFSET	
		I			HARD	SANDY LEAN CLAY WIT	H GRAVEL, MOIST							1	7' EAST DUE TO O TREES	
		2	Å.		иси у	7 ₩₽₮₩₩₽₽₽₽ \$₮₦₽\$₩	NE CDINE IV							< 		
5	.0 - 3	3	X			WENINEKED SKNDSIO	NE OKADE IV							1	Į0	
	'	1	Î		RUN 1	-6.5' TO 12' ERY = 87.3% ROD = 0'					-		-+		TRANSITIONAL BEDROCK MAY	
		6			FRESH	SANDSTONE									INCLUDE LARGE DISLODGED ROCK	
10	.0 0.														SLABS WITH SOIL INFILLS	
	-		H		RUN 2	- 12' TO 17'	20/								DUE TO WEATHERING	
15		7			FRESH	SANDSTONE	J70									
10	.0 -				•										AVERAGE ROCK Coring rate =	
_	-		IT		RUN 3 RECOVE	$\frac{1}{-17'} \frac{1}{10} \frac{1}{22'} \frac{1}{-10} \frac{1}{22'} \frac{1}{22'} \frac{1}{-10} \frac{1}{22'} \frac{1}$				+			-+	- +	3 MINUTES PER FOOT	
11S.CP	.0 - 0.	B	П		COMPL	ETELY WEATHERED SAN	NDSTONE GRADE V									
8 POIN					· ·											
(TERA	-				RUN 4 RECOVI	- 22' TO 27' ERY = 84%. RQD = 25%	%									
an 55 25	.0 - 9	9			IKF2H	SANDSTUNE										
/2017-0					DIIN S	27' ጥቢ 22'										
JECTS					RECOVE	ERY = 98%. RQD = 35. SANDSTONE	8%									
30 30	.0 - 1	0		•	FRESH	CONGLOMERATE										
36)/GIN	_				FRESH RUN 6	SANDSTONE - 32' TO 37'										
LES (X	1	1			RECOVI	ERY = 80%. RQD = 30.3 SANDSTONE										
	.0 - 1	-			:											
PROG	-				RUN 1	- 37' TO 40.5'	5%								WATER RETURN	
18 - C:		2			FRESH	SANDSTONE	s/u								LOSS OF WATER RETURN AT 38.5'	
/17 14:	-					End of Borin	ıg @ 40.5 feet			1	1	I		1		
от - 9/1	-															
19. 20 19.	.0 _			L												
2 Calibra 5 Calibra 6 Calibra 7 Cali												BORING S	TARTED			
- LOG	DATE			TI	ME	SAMPLED	CAVE-	IN WATER BORING COMPLETED)/17		
NGN00	6/20/17			10	:00	BCR	40	-		35.5		6/21 DRILLER RIG			/17	
0 6 /	6/21/17 11:00 ACR - 5.4									DRY		DRAWN B	Y	PW APPR	CIVIE 550 Oved	
2													J. AL	LLN		







Ke	nn	ley	G	e	otec	hnical Services	5	Job No. 2017-054									
CLIEN	CLIENT 8 POINTS WIND, LLC										EIGHT	EIGHT POINTS WIND ENERGY PROJECT					
ENGIN	ENGINEER SARGENT AND LUNDY, LLC										STEUE	EN COU	NTY, NY	Z			
DEPTH		SAMPLE NO.	SAMPLER TYPE	RECOVERY	GRAPHIC LOG	BORING NUMBER SURFACE ELEVATION LATITUDE (degrees) 42.009269	T-30 Sheet LONGITUDE (degree 77.	1 of 1 2237.7 ²⁵⁾ .658621		Uncor	nfined Comp	ressive Strei LUF blows /	ngth, tons/ft. 4 LL	5		REMARKS	
						DESCRIPTION (LABORATORY	N OF MATERIALS CLASSIFICATION)			10	20	30 40 50					
5	-	1 2 3				GLACIAL TILL CONSISTING OF SAND WITH SILT, MOIST GLACIAL TILL CONSISTING OF CLAY WITH GRAVEL, MOIST	BROWN VERY DENSE (GRAVELLY							100 - 100 ↓ 100 ↓ 100	DRILLER NOTES) POSSIBLE COBBLES AND BOULDERS	
10	-	4 5	X			HIGHLY WEATHERED SHALE G	RADE IV								10(◇ 10(◇)	
15	- - 0.0	6				RUN 1 - 12' TO 17' RECOVERY = 82%. RQD = 37 FRESH SANDSTONE	.5%]	TRANSITIONAL BEDROCK MAY INCLUDE LARGE DISLODGED ROCK SLABS WITH SOIL INFILLS DUE EC	
	- 1.0	7				RUN 2 - 17' TO 22' RECOVERY = 54%. RQD = 0% FRESH SHALE	6									DUE TU WEATHERING LOSS OF WATER RETURN AT 19'	
717-054 NEX IEKA	-	8				RUN 3 - 22' TO 27' RECOVERY = 100%. RQD = 8 FRESH SANDSTONE	6.6%								!	590 GALLONS OF WATER USED WHILE ROCK CORING	
	-	9				RUN 4 - 27' TO 32' RECOVERY = 100%. RQD = 5 FRESH SANDSTONE	5.8%									AVERAGE ROCK CORING RATE = 3.7 MINUTES PER FOOT	
JGRAM FILES (X86	.0 —	10				RECOVERY $= 96\%$ RQD $= 70$ FRESH SANDSTONE	.8%										
9/1/1/ 14:48 - C:\PK	- .0 -	11				RECOVERY = 43%. RQD = 13 FRESH SANDSTONE. End of Bor	.8% ing @ 40 feet										
- 105.50N	.0 _									- <u> </u>							
						WATER LEVEL	MEASUREMENTS	•	vaiwidted	T EUGLIOUIIG[BORING ST	TARTED	n 		/17		
- FOC	DATI	E		TIME SAMPLED CASING CAV						WAT	ER	BORING C	OMPLETED		5/30/ 5/21	/11	
5/ 5/	5/31/17 5/31/17				09: 09:	00 BCR 30 ACR	40 -	- 7		18. DR	.1 Y	DRILLER	v	KGES	RIG	7822DT	
.90												DKHMN R	D. I	LLEN	APPKU	ለ ተገ	

Ken	ney	Ge	eot	ec	hnica	al Services	5								Job No. 2017-054		
CLIENT	CLIENT 8 POINTS WIND, LLC										EIGHT POINTS WIND ENERGY PROJECT						
ENGINEE	ENGINEER SARGENT AND LUNDY, LLC										STEUE	STEUBEN COUNTY, NY					
					BORING NUMBER T-31 Sheet 1 of 1					Unconf							
HL	E NO.	R TYPE	VLKI	IC FOR	SURFACI	E ELEVATION		2261.0		1	2	3	4	5	T IKK		
DEI	SAMPI	SAMPLE	UUL KEUU	ылаги	LATITUD	DE (degrees) 42.015262	LONGITUDE (degree 77	es) . 638421		PL X	N VA		<u>LL</u>		REM		
						DESCRIPTION (LABORATORY	OF MATERIALS CLASSIFICATION)			10	20		40	50			
	1	Χ			BROWN	STIFF SANDY SILT, M	OIST										
	2				GLACIAI WITH G	TILL CONSISTING OF RAVEL, MOIST	BROWN HARD SANDY	SILT	_					*	DRILLER NOTES POSSIBLE CORBLES AND		
5.0 -	- 3	X			GLACIAI CLAY W	TILL CONSISTING OF TH GRAVEL, MOIST	RED-BROWN HARD SA	NDY LEAN							BOULDERS		
	4	Å			WEATHE	. 80' TO 125'	DE IV — — — — — — — —						_+				
10.0 -	5			· · · · · · · · · · · · · · · · · · ·	RECOVE FRESH S	RY = 58%. RQD = 24% SANDSTONE	<i>∀</i> ₀								BEDROCK MAY INCLUDE LARGE DISLODGED ROCK SLABS WITH		
15.0 -	6	h			RUN 2 - RECOVE FRESH S	- 12.5' TO 16.2' RY = 54%. RQD = 0% SANDSTONE									SOIL INFILLS DUE TO WEATHERING		
G	- 7	H		· · · ·	NATURA RUN 3 - RECOVE	LL DISCONTINUITY AT · 16.2' TO 20.5' RY = 81%. RQD = 21. SANDSTONE	13.0' AND CLAY INFIL	L AT 14.0'							CORING RATE = 3-4 MINUTES PER FOOT		
9.STNIO 20.0 -	_				HIGHLY	WEATHERED SHALE G	RADE IV	/									
VEXTERA 8	8				RUN 4 - RECOVE	~ 20.5' TO 25.5' RY = 80%. RQD = 25% SANDSTONE SHALE	/0	7									
1 25.0 - 2 - 024 -	1	Н			HIGHLY	WEATHERED SHALE G	RADE IV								_		
-015	9			· · · · · · · · · · · · · · · · · · ·	RUN 5 - RECOVE FRESH S	25.5° TU 30.5° RY = 84%. RQD = 339 SANDSTONE	∕₀										
S (X86)/GINT/P	- 10			· · · ·	RUN 6 - RECOVE FRESH S	- 30.5' TO 35.5' RY = 64%. RQD = 15% SANDSTONE	∕₀										
35.0 -	-	╢		· · · ·	RUN 7 -	35.5' TO 40.5'											
NPROG	11				FRESH S	RY = 70%. RQD = 0% SANDSTONE SHALE	1	7							_		
- 0.04	-	ון		· · · ·	FRESH S	SANDSTONE Fnd of Bovin	ur @ 405 feet	/							_		
r - 9/1/17	-						-y - 1910 1001										
LGD:E00-45.0 -										Calibrated P	 'enetromet(er Unconfine	 l Compression				
G A G	WATER LEVEL MEASUREMENTS											BORING ST	FARTED	7/1	1/17		
AD 23-FO	TE			TIM	E	SAMPLED	CASING	CAVE-	IN	WATE	R	BORING C	OMPLETED	7/1	1/17		
	11/7/17		1	17:3	30 No	BCR	40.5	-	,	DRY	Y v	DRILLER		PW RIG	CMF 550		
Y 11/1	1/11			10:1	10	АСЛ	-	31.4	<u> </u>	מאמ	L	DRAWN B	MOREHO	DUSE APP	ROVED		
							I	1									





-OG A GNGN03 - LOG A GNGN03.GDT - 9/1/17 15:16 - C: PROGRAM FILES (X86)/GINTIPROJECTS\2017-054 NEXTERA 8 POINTS.GPJ







-OG A GNGN03 - LOG A GNGN03.GDT - 9/1/17 15:16 - C:PROGRAM FILES (X86)/GINT/PROJECTS/2017-054 NEXTERA 8 POINTS.GPJ



















STD US LAB.GDT - 9/1/17 14:55 - C:\PROGRAM FILES (X86)\GINT\PROJECTS\2017-054 NEXTERA 8 POINTS.GPJ



STD US LAB.GDT - 9/1/17 14:55 - C:\PROGRAM FILES (X86)\GINT\PROJECTS\2017-054 NEXTERA 8 POINTS.GPJ



STD US LAB.GDT - 9/1/17 14:56 - C:\PROGRAM FILES (X86)\GINT\PROJECTS\2017-054 NEXTERA 8 POINTS.GPJ GINT















STD US LAB.GDT - 9/1/17 14:59 - C:\PROGRAM FILES (X86)\GINT\PROJECTS\2017-054 NEXTERA 8 POINTS.GPJ



STD US LAB.GDT - 8/29/17 15:12 - C:\PROGRAM FILES (X86)\GINT\PROJECTS\2017-054 NEXTERA 8 POINTS.GPJ



STD US LAB.GDT - 9/1/17 15:00 - C:\PROGRAM FILES (X86)\GINT\PROJECTS\2017-054 NEXTERA 8 POINTS.GPJ







STD US LAB.GDT - 9/1/17 15:01 - C:\PROGRAM FILES (X86)\GINT\PROJECTS\2017-054 NEXTERA 8 POINTS.GPJ


STD US LAB.GDT - 9/1/17 15:01 - C:\PROGRAM FILES (X86)\GINT\PROJECTS\2017-054 NEXTERA 8 POINTS.GPJ









STD US LAB.GDT - 8/29/17 15:14 - C:\PROGRAM FILES (X86)\GINT\PROJECTS\2017-054 NEXTERA 8 POINTS.GPJ









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A CARDON					New Average	and the second				1 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 -			*	
														-
BORING	RUN	START	FINISH	RECOVERY	RQD		8		STR	RENGTH	TESTING		-	
								DEPTH TO	DIAMETER	LENGT		AREA	LOAD	ΣcUNCONFINE
T-4	1	10.0	15.0	100%	0.0%	BORING	RUN	TOP OF SAMPLE (FT.)	(IN.)	H (IN.)	L/D	(SQ.IN.)	(LBS)	COMPRESSI STRENGTH (
T-4	2	15.0	20.0	100%	6.7%	800 B								
T-4	3	20.0	25.0	94%	46.7%	RUN	2 3	22.1	1.97	4.43	2.25	3.05	34,740	11,380
T-4	4	25.0	30.0	84%	0.0%	1			/					
Destation	0				to the second		Steel .		/			9	10	A CONTRACTOR
Called Martin	0 0	Company side of	-		A	-	-			-	-	0		
	-11	UT									AN	Trang		
	RU	JN 2		TFF	C CA	It				F		E		
	C	RUN 3										E		
(Trap	R	UN 4						and a				TIT
N. S. C. S.	Lan market	S - Anton	all and the second	and the second s			1					10000	-	















			1				-		Z									
		- Ale			KOBALT.	7												
BORING	IG RUN START FINISH RECOVERY RQD							STRENGTH TESTING										
T-11	1	20.0	25.0	80%	38.3%	Doort	D	EPTH TO TOP	DIAMETER	LENGTH		AREA	LOAD					
T-11	2	25.0	30.0	84%	30%	BORIN	GRUN	(FT.)	(IN.)	(IN.)	L/D	(SQ.IN.)	(LBS)	STRENGTH (PSI)				
T-11	3	30.0	35.0	40%	0%	T-11	1	20.1	1.97	4.42	2.24	3.04	36,240	11,920				
T-11	4	35.0	40.0	40%	0.0%	1.	100	2	T.				1	a star				
37164		J		12. 				1	·	-	*	Ó	-	-				
-11 OP		-				Carlos y			F			-						
UN 2	State of Lot of				L Gart		7 -	- 27		5								
JN 3					RUN 4	19-18							E	т-11 воттом				
1 and				1 m		1	NA	TURAL	ISCONT	INUITI	ES WI	THCL	AY INF	ILL				













	AT			Ma IN.	Y W		2				1			I	
ROPING	DIN	START		PECOVERY	POD					- with					
T_19	1	15 0	20.0	7/%	19.3%	STRENGTH TESTING									
T-10	2	20.0	20.0	100%	43.3%	-			DEPTH TO TOP				10000	10000	σ. UCONFINED
T-18	3	25.0	30.0	50%	8 3%		BORING	RUN	OF SAMPLE	DIA METER	LENGTH (IN)	L/D	AREA (SO IN)		COMPRESSIVE
T-18	4	30.0	35.0	50%	10.0%				(FT.)	()	(int)		(Sugarity	(LDJ)	STRENGTH (PSI)
T-18	5	35.0	40	58%	44.2%	-	T-18	1	15.8	1.86	4.18	2.25	2.72	20,700	7,610
T-18	Kar		-			-	-	-		-		17	r		1
TOP		Ale.	19860	Carlo V		-		-		RUN	2	14			Contract P
-9	1 16-	N. all	TORES	RUN 4	16			TAC		RUN 3	all a				
· brand	to be and	A CALL	Jac d		रात)			2	YA A	A A			- 32	the bar	and the second second
RUN 5		-		h is an			K- 16		T-18 BOTTOM						-
	1					T									
1. 1		No The				2			Le War		MAR I		10	1	No Maria


























Geotechnical Parameters for Deep Foundations - Transmission Line Structures											
Boring	TL	3	Ultir	nate	Allov	vable	Total S	tress	Static		
				End		End	Internal		Lateral		MFAD
			Skin	Bearing	Skin	Bearing	Angle of		Subgrade		Deformation
			Friction	Pressure	Friction	Pressure	Friction ϕ	Cohesion,	Modulus,		Modulus E _D
Soil	Depth	Avg N	(psf)	(psf)	(psf)	(psf)	(degrees)	c (psf)	k (pci)	Strain ϵ_{50}	(ksi)
Sandy ML-CL	0-4	14	800	ignore	400	ignore	-	1750	500	0.007	1
Sandy ML-CL	4'-7'	16	1000	12000	500	6000	-	2000	500	0.007	1.3
Gravelly CL	7'-13'	47	2000	30000	1000	15000	-	5000	1000	0.005	3.5
Sandy ML-CL	13'-18'	32	1500	24000	750	12000	-	4000	1000	0.005	2.5
Sandy MIL-CL	18-25	88	1500	36000	750	18000	-	6000	2000	0.004	/
Boring	TL-4		Ultimate		Allowable		Total Stress		Static		
				End		End	Internal		Lateral		MFAD
			Skin	Bearing	Skin	Bearing	Angle of		Subgrade		Deformation
			Friction	Pressure	Friction	Pressure	Friction 	Cohesion,	Modulus,		Modulus E _D
Soil	Depth	Avg N	(psf)	(psf)	(psf)	(psf)	(degrees)	c (psf)	k (pci)	Strain ϵ_{50}	(ksi)
Sandy ML	0-4	8	ignore	ignore	ignore	ignore	30	ignore	25	-	0.8
Sandy ML	4'-10'	52	1000	12000	500	6000	40	-	275	-	3
					Rock Mass R		ating (RMR76)				MFAD
											Deformation
											Modulus ED
Rock	Depth	qu (ksi)	RQD	qu (ksi)	RQD	Jt Space	Jt Cond	GW	Orient	RMR76	(ksi)
Sandstone	10'-25'	1.5	0	1	3	5	0	7	-7	9	42
Boring	TL	9	Ultimate		Allowable		Total Stress		Static		
				End		End	Internal		Lateral		MFAD
			Skin	Bearing	Skin	Bearing	Angle of		Subgrade		Deformation
			Friction	Pressure	Friction	Pressure	Friction ϕ	Cohesion,	Modulus,		Modulus E _D
Soil	Depth	Avg N	(psf)	(psf)	(psf)	(psf)	(degrees)	c (psf)	k (pci)	Strain $\epsilon_{\scriptscriptstyle 50}$	(ksi)
Sandy ML-CL	0-4	14	800	ignore	400	ignore	-	1750	500	0.007	1
						-					
Sandy ML-CL	4'-8'	12	650	9000	325	4500	-	1500	500	0.007	0.9
Sandy ML-CL Sandy ML-CL	4'-8' 8'-18'	12 42	650 1000	9000 30000	325 500	4500 15000	-	1500 5000	500 1000	0.007 0.005	0.9 2.5
Sandy ML-CL Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25'	12 42 100	650 1000 1500	9000 30000 36000	325 500 750	4500 15000 18000	- - -	1500 5000 6000	500 1000 2000	0.007 0.005 0.004	0.9 2.5 7
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring	4'-8' 8'-18' 18'-25' TL·	12 42 100	650 1000 1500 Ultir	9000 30000 36000 mate	325 500 750 Allov	4500 15000 18000 vable	- - - Total S	1500 5000 6000	500 1000 2000 Static	0.007 0.005 0.004	0.9 2.5 7
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring	4'-8' 8'-18' 18'-25' TL	12 42 100 10	650 1000 1500 Ultir	9000 30000 36000 nate End	325 500 750 Allov	4500 15000 18000 vable End	- - - Total S Internal	1500 5000 6000 Stress	500 1000 2000 Static Lateral	0.007 0.005 0.004	0.9 2.5 7 MFAD
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring	4'-8' 8'-18' 18'-25' TL-	12 42 100	650 1000 1500 Ultir Skin	9000 30000 36000 mate End Bearing	325 500 750 Allov Skin	4500 15000 18000 vable End Bearing	- - - Total S Internal Angle of	1500 5000 6000 Stress	500 1000 2000 Static Lateral Subgrade	0.007 0.005 0.004	0.9 2.5 7 MFAD Deformation
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring	4'-8' 8'-18' 18'-25' TL	12 42 100	650 1000 1500 Ultir Skin Friction	9000 30000 36000 nate End Bearing Pressure	325 500 750 Allov Skin Friction	4500 15000 18000 vable End Bearing Pressure	- - - Internal Angle of Friction ϕ	1500 5000 6000 itress Cohesion,	500 1000 2000 Static Lateral Subgrade Modulus,	0.007 0.005 0.004	0.9 2.5 7 MFAD Deformation Modulus E _D
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring Soil	4'-8' 8'-18' 18'-25' TL- Depth	12 42 100 10 Avg N	650 1000 1500 Ultir Skin Friction (psf)	9000 30000 36000 nate End Bearing Pressure (psf)	325 500 750 Allov Skin Friction (psf)	4500 15000 18000 vable End Bearing Pressure (psf)	- - - Internal Angle of Friction ¢ (degrees)	1500 5000 6000 itress Cohesion, c (psf)	500 1000 2000 Static Lateral Subgrade Modulus, k (pci)	0.007 0.005 0.004 Strain ε ₅₀	0.9 2.5 7 MFAD Deformation Modulus E _D (ksi)
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML	4'-8' 8'-18' 18'-25' TL- Depth 0-2.5	12 42 100 •10 •10 • • •	650 1000 1500 Ultir Skin Friction (psf) ignore	9000 30000 36000 mate End Bearing Pressure (psf) ignore	325 500 750 Allov Skin Friction (psf) ignore	4500 15000 18000 vable End Bearing Pressure (psf) ignore	- - - Internal Angle of Friction ¢ (degrees) 30	1500 5000 6000 itress Cohesion, c (psf) ignore	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25	0.007 0.005 0.004 Strain ε ₅₀	0.9 2.5 7 MFAD Deformation Modulus E _D (ksi) 0.8
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML Sandy ML-CL	4'-8' 8'-18' 18'-25' TL- Depth 0-2.5 2.5'-4'	12 42 100 10 Avg N 8 12	650 1000 1500 Ultir Skin Friction (psf) ignore 650	9000 30000 36000 mate End Bearing Pressure (psf) ignore ignore	325 500 750 Allov Skin Friction (psf) ignore 325	4500 15000 18000 vable End Bearing Pressure (psf) ignore ignore	- - - Internal Angle of Friction ¢ (degrees) 30 -	1500 5000 6000 itress Cohesion, c (psf) ignore 1500	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500	0.007 0.005 0.004 Strain ε ₅₀ - 0.007	0.9 2.5 7 MFAD Deformation Modulus E _p (ksi) 0.8 0.9
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25' TL- Depth 0-2.5 2.5'-4' 4'-6'	12 42 100 10 Avg N 8 12 26	650 1000 1500 Skin Friction (psf) ignore 650 1000	9000 30000 36000 mate End Bearing Pressure (psf) ignore ignore 19200	325 500 750 Allov Skin Friction (psf) ignore 325 500	4500 15000 18000 vable End Bearing Pressure (psf) ignore ignore 9600	- - - Internal Angle of Friction ¢ (degrees) 30 - -	1500 5000 6000 itress Cohesion, c (psf) ignore 1500 3200	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500 1000	0.007 0.005 0.004 Strain ε ₅₀ - 0.007 0.005	0.9 2.5 7 MFAD Deformation Modulus E _p (ksi) 0.8 0.9 2
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML Sandy ML-CL Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25' TL- Depth 0-2.5 2.5'-4' 4'-6' 6'-10' 10.35'	12 42 100 10 Avg N 8 12 26 51	650 1000 1500 Ultin Friction (psf) ignore 650 1000 2000	9000 30000 36000 mate End Bearing Pressure (psf) ignore ignore 19200 30000	325 500 750 Allov Skin Friction (psf) ignore 325 500 1000 500	4500 15000 18000 vable End Bearing Pressure (psf) ignore ignore 9600 15000 0600	- - - - - - - - - - - -	1500 5000 6000 itress Cohesion, c (psf) ignore 1500 3200 5000	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500 1000 1000	0.007 0.005 0.004 Strain ε ₅₀ - 0.007 0.005 0.005	0.9 2.5 7 MFAD Deformation Modulus E _D (ksi) 0.8 0.9 2 3.5
Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25' TL- Depth 0-2.5 2.5'-4' 4'-6' 6'-10' 10-25'	12 42 100 10 Avg N 8 12 26 51 27	650 1000 1500 Ultir Skin Friction (psf) ignore 650 1000 2000 1000	9000 30000 36000 mate End Bearing Pressure (psf) ignore ignore 19200 30000 19200	325 500 750 Skin Friction (psf) ignore 325 500 1000 500	4500 15000 18000 vable End Bearing Pressure (psf) ignore ignore 9600 15000 9600	- - Total S Internal Angle of Friction ¢ (degrees) 30 - - - -	1500 5000 6000 itress Cohesion, c (psf) ignore 1500 3200 5000 3200	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500 1000 1000	0.007 0.005 0.004 Strain ε ₅₀ - 0.007 0.005 0.005	0.9 2.5 7 MFAD Deformation Modulus E _D (ksi) 0.8 0.9 2 3.5 2
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25' TL- Depth 0-2.5 2.5'-4' 4'-6' 6'-10' 10-25' TL-	12 42 100 10 Avg N 8 12 26 51 27 26	650 1000 1500 Ultir Skin Friction (psf) ignore 650 1000 2000 1000	9000 30000 36000 mate End Bearing Pressure (psf) ignore ignore 19200 30000 19200	325 500 750 Allov Skin Friction (psf) ignore 325 500 1000 500 Allov	4500 15000 18000 vable End Bearing Pressure (psf) ignore ignore 9600 15000 9600 vable	- Total S Internal Angle of Friction ¢ (degrees) 30 - - - - - - - - - - - -	1500 5000 6000 itress Cohesion, c (psf) ignore 1500 3200 5000 3200 itress	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500 1000 1000 1000 Static	0.007 0.005 0.004 Strain ε ₅₀ - 0.007 0.005 0.005 0.005	0.9 2.5 7 MFAD Deformation Modulus E _p (ksi) 0.8 0.9 2 3.5 2
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25' TL- Depth 0-2.5 2.5'-4' 4'-6' 6'-10' 10-25' TL-	12 42 100 10 Avg N 8 12 26 51 27 26	650 1000 1500 Ultir Skin Friction (psf) ignore 650 1000 2000 1000 Ultir	9000 30000 36000 nate End Bearing Pressure (psf) ignore ignore 19200 30000 19200 mate End	325 500 750 Allov Skin Friction (psf) ignore 325 500 1000 500 Allov	4500 15000 18000 vable End Bearing Pressure (psf) ignore ignore 9600 15000 9600 vable End	- Total S Internal Angle of Friction ¢ (degrees) 30 - - - - - - Total S Internal	1500 5000 6000 itress Cohesion, c (psf) ignore 1500 3200 5000 3200 itress	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500 1000 1000 1000 1000 Static Lateral	0.007 0.005 0.004 Strain ε ₅₀ - 0.007 0.005 0.005 0.005	0.9 2.5 7 MFAD Deformation Modulus E _p (ksi) 0.8 0.9 2 3.5 2 3.5 2
Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25' TL- Depth 0-2.5 2.5'-4' 4'-6' 6'-10' 10-25' TL-	12 42 100 10 Avg N 8 12 26 51 27 26	650 1000 1500 Skin Friction (psf) ignore 650 1000 2000 1000 Ultir Skin	9000 30000 36000 mate End Bearing Pressure (psf) ignore ignore 19200 30000 19200 mate End Bearing	325 500 750 Skin Friction (psf) ignore 325 500 1000 500 Allov Skin	4500 15000 18000 vable End Bearing Pressure (psf) ignore ignore 9600 15000 9600 vable End Bearing	- Total S Internal Angle of Friction ¢ (degrees) 30 - - - - - Total S Internal Angle of	1500 5000 6000 itress Cohesion, c (psf) ignore 1500 3200 5000 3200 itress	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500 1000 1000 1000 1000 Static Lateral Subgrade	0.007 0.005 0.004 Strain ε ₅₀ - 0.007 0.005 0.005	0.9 2.5 7 MFAD Deformation Modulus E _p (ksi) 0.8 0.9 2 3.5 2 3.5 2 MFAD Deformation
Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25' TL- Depth 0-2.5 2.5'-4' 4'-6' 6'-10' 10-25' TL-	12 42 100 10 Avg N 8 12 26 51 27 26	650 1000 1500 Skin Friction (psf) ignore 650 1000 2000 1000 Ultir Skin Friction	9000 30000 36000 mate End Bearing Pressure (psf) ignore ignore 19200 30000 19200 mate End Bearing Pressure	325 500 750 Skin Friction (psf) ignore 325 500 1000 500 Allov Skin Friction	4500 15000 18000 vable End Bearing Pressure (psf) ignore ignore 9600 15000 9600 vable End Bearing Pressure	- Total S Internal Angle of Friction ¢ (degrees) 30 - - - - - Total S Internal Angle of Friction ¢	1500 5000 6000 itress Cohesion, c (psf) ignore 1500 3200 5000 3200 5000 3200 5000 3200	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500 1000 1000 1000 1000 Static Lateral Subgrade Modulus,	0.007 0.005 0.004 Strain ε ₅₀ - 0.007 0.005 0.005	0.9 2.5 7 MFAD Deformation Modulus E _D (ksi) 0.8 0.9 2 3.5 2 3.5 2 MFAD Deformation Modulus E _D
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25' TL Depth 0-2.5 2.5'-4' 4'-6' 6'-10' 10-25' TL Depth	12 42 100 10 Avg N 8 12 26 51 27 26 27 26	650 1000 1500 Skin Friction (psf) ignore 650 1000 2000 1000 Ultir Skin Friction (psf)	9000 30000 36000 mate End Bearing Pressure (psf) ignore ignore ignore 19200 30000 19200 19200 mate End Bearing Pressure (psf)	325 500 750 Skin Friction (psf) ignore 325 500 1000 500 Skin Friction (psf)	4500 15000 18000 vable End Bearing Pressure (psf) ignore ignore 9600 15000 9600 vable End Bearing Pressure (psf)	- Total S Internal Angle of Friction ¢ (degrees) 30 - - - - Total S Internal Angle of Friction ¢ (degrees)	1500 5000 6000 itress Cohesion, c (psf) ignore 1500 3200 5000 3200 itress Cohesion, c (psf)	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500 1000 1000 1000 1000 1000 Static Lateral Subgrade Modulus, k (pci)	0.007 0.005 0.004 Strain ε ₅₀ - 0.007 0.005 0.005 0.005 Strain ε ₅₀	0.9 2.5 7 MFAD Deformation Modulus E _p (ksi) 0.8 0.9 2 3.5 2 3.5 2 MFAD Deformation Modulus E _p (ksi)
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring Sandy ML Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25' TL- Depth 0-2.5 2.5'-4' 4'-6' 6'-10' 10-25' TL- Depth 0-4	12 42 100 10 Avg N 8 12 26 51 27 26 27 26 4vg N 19	650 1000 1500 VItin Friction (psf) ignore 650 1000 2000 1000 Ultin Skin Friction (psf) 900	9000 30000 36000 mate End Bearing Pressure (psf) ignore 19200 30000 19200 mate End Bearing Pressure (psf) ignore 24000	325 500 750 Allov Skin Friction (psf) ignore 325 500 1000 500 250 Allov Skin Friction (psf) 450 750	4500 15000 18000 vable End Bearing Pressure (psf) ignore 9600 15000 9600 vable End Bearing Pressure (psf) ignore 12000	- Total S Internal Angle of Friction ¢ (degrees) 30 - - - - - - - - - - - - - - - - - -	1500 5000 6000 itress Cohesion, c (psf) 3200 3200 3200 3200 3200 3200 stress Cohesion, c (psf) 2200	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500 1000 1000 1000 1000 Static Lateral Subgrade Modulus, k (pci) 500	0.007 0.005 0.004 Strain ε ₅₀ - 0.007 0.005 0.005 Strain ε ₅₀ 0.007	0.9 2.5 7 MFAD Deformation Modulus E _D (ksi) 0.8 0.9 2 3.5 2 MFAD Deformation Modulus E _D (ksi) 1.5 2.5
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25' TL- Depth 0-2.5 2.5'-4' 4'-6' 6'-10' 10-25' TL- Depth 0-4 4'-14'	12 42 100 10 Avg N 8 12 26 51 27 26 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 3 2 3 2 3 2 3 2 3 2 3 3 2 3 3 2 3 3 4 3 4 3 4 4 4 4 5 1 1 1 1 1 1 1 1 1 1	650 1000 1500 Ultin Skin Friction (psf) 1000 2000 1000 Ultin Skin Friction (psf) 900 1500	9000 30000 36000 mate End Bearing Pressure (psf) ignore 19200 30000 19200 mate End Bearing Pressure (psf) ignore 24000 25200	325 500 750 Skin Friction (psf) ignore 325 500 1000 500 Skin Friction (psf) 450 750 750	4500 15000 18000 vable End Bearing Pressure (psf) ignore 9600 15000 9600 vable End Bearing Pressure (psf) ignore 15000 9600 15000 9600 15000 9600 150000 150000 15000	- - - - - - - - - - - - - -	1500 5000 6000 itress Cohesion, c (psf) 3200 3200 3200 3200 3200 itress Cohesion, c (psf) 2200 4000 4200	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500 1000 1000 1000 Static Lateral Subgrade Modulus, k (pci) 500 1000	0.007 0.005 0.004 Strain ε ₅₀ 0.007 0.005 0.005 Strain ε ₅₀ 0.007 0.005	0.9 2.5 7 MFAD Deformation Modulus E _D (ksi) 0.8 0.9 2 3.5 2 MFAD Deformation Modulus E _D (ksi) 1.5 2.5 2.9
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25' TL- Depth 0-2.5 2.5'-4' 4'-6' 6'-10' 10-25' TL- 0-4 4'-14' 10-25' Depth 0-4 4'-14' 14'-20' 20'-25'	12 42 100 10 Avg N 8 12 26 51 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 27 26 27 26 27 27 26 27 27 26 27 27 26 27 27 26 27 27 27 26 27 27 26 27 27 26 27 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 27 26 27 26 27 26 27 27 26 27 27 26 27 27 26 27 27 26 27 27 27 26 27 27 27 26 27 27 27 27 26 27 27 27 27 27 27 27 27	650 1000 1500 Ultir Skin Friction (psf) ignore 650 1000 2000 1000 2000 1000 Ultir Skin Friction (psf) 900 1500 1500	9000 30000 36000 mate End Bearing Pressure (psf) ignore ignore 19200 30000 19200 mate End Bearing Pressure (psf) ignore 24000 25200 36000	325 500 750 Skin Friction (psf) ignore 325 500 1000 500 Allov Skin Friction (psf) 450 750 750 750	4500 15000 18000 vable End Bearing Pressure (psf) ignore ignore 9600 15000 9600 vable End Bearing Pressure (psf) ignore 12000 12600 18000	- - - - - - - - - - - - - -	1500 5000 6000 itress Cohesion, c (psf) ignore 1500 3200 5000 3200 itress Cohesion, c (psf) 2200 4000 4200 6000	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500 1000 1000 1000 Static Lateral Subgrade Modulus, k (pci) 500 1000 1000	0.007 0.005 0.004 Strain ε ₅₀ - 0.007 0.005 0.005 0.005 0.007 0.005 0.005 0.005	0.9 2.5 7 MFAD Deformation Modulus E _p (ksi) 0.8 0.9 2 3.5 2 MFAD Deformation Modulus E _p (ksi) 1.5 2.5 2.9 7
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25' Depth 0-2.5 2.5'-4' 4'-6' 6'-10' 10-25' TL - 0-25' Depth 0-4 4'-14' 14'-20' 20'-25'	12 42 100 10 Avg N 8 12 26 51 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 5127 5127 26 51271271271121111111111111	650 1000 1500 Ultir Skin Friction (psf) ignore 650 1000 2000 1000 2000 1000 Ultir Skin Friction (psf) 900 1500 1500	9000 30000 36000 mate End Bearing Pressure (psf) ignore 19200 30000 19200 mate End Bearing Pressure (psf) ignore 24000 25200 36000	325 500 750 Skin Friction (psf) ignore 325 500 1000 500 Allov Skin Friction (psf) 450 750 750	4500 15000 18000 vable End Bearing Pressure (psf) ignore 9600 15000 9600 vable End Bearing Pressure (psf) ignore 12000 12600 18000	- Total S Internal Angle of Friction ϕ (degrees) 30 - - - - - - - - - - - - - - - - - -	1500 5000 6000 itress Cohesion, c (psf) ignore 1500 3200 5000 3200 itress Cohesion, c (psf) 2200 4000 4200 6000	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500 1000 1000 1000 1000 Static Lateral Subgrade Modulus, k (pci) 500 1000	0.007 0.005 0.004 Strain ε ₅₀ - 0.007 0.005 0.005 0.005 Strain ε ₅₀ 0.007 0.005 0.007	0.9 2.5 7 MFAD Deformation Modulus E _p (ksi) 0.8 0.9 2 3.5 2 3.5 2 3.5 2 MFAD Deformation Modulus E _p (ksi) 1.5 2.5 2.9 7
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25' TL- Depth 0-2.5 2.5'-4' 4'-6' 6'-10' 10-25' TL- Depth 0-4 4'-14' 14'-20' 20'-25'	12 42 100 10 Avg N 8 12 26 51 27 26 51 27 26 27 26 27 26 27 26 27 26 27 26 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 27 26 27 26 27 27 26 27 27 26 27 26 27 27 26 27 27 26 27 27 26 27 27 26 27 27 26 27 27 27 26 27 27 27 26 27 27 27 27 26 27 27 27 27 26 27 27 27 27 26 27 27 27 26 2727 27 27 27 27 27 27	650 1000 1500 Skin Friction (psf) ignore 650 1000 2000 1000 2000 1000 Ultir Skin Friction (psf) 900 1500 1500	9000 30000 36000 mate End Bearing Pressure (psf) ignore ignore 19200 30000 19200 mate End Bearing Pressure (psf) ignore 24000 25200 36000 mate End	325 500 750 Skin Friction (psf) ignore 325 500 1000 500 Skin Friction (psf) 450 750 750 750 750 750	4500 15000 18000 vable End Bearing Pressure (psf) ignore 9600 15000 9600 vable End Bearing Pressure (psf) ignore 12000 12600 18000 vable	- - - - - - - - - - - - - -	1500 5000 6000 itress Cohesion, c (psf) ignore 1500 3200 5000 3200 5000 3200 5000 3200 5000 3200 5000 3200 5000 3200 5000 3200 5000 3200 5000 3200 5000 3200 5000 50	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500 1000 1000 1000 1000 1000 Static Lateral Subgrade Modulus, k (pci) 500 1000 1000 2000	0.007 0.005 0.004 Strain ε ₅₀ 0.005 0.005 0.005 0.007 0.005 0.005 0.005 0.005	0.9 2.5 7 MFAD Deformation Modulus E _p (ksi) 0.8 0.9 2 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25' TL - 0-2.5 2.5'-4' 4'-6' 6'-10' 10-25' TL - 0-4 4'-14' 14'-20' 20'-25' TL -	12 42 100 10 Avg N 8 12 26 51 27 26 51 27 26 27 26 27 26 27 26 27 26 27 26 5 1 27 27 26 5 1 27 26 5 1 27 27 26 5 1 27 26 5 1 27 27 26 5 1 27 27 26 5 1 27 27 26 5 1 27 27 26 5 1 27 27 26 5 1 27 27 26 5 1 27 27 26 5 1 27 26 5 1 27 27 26 27 27 26 27 27 26 27 27 26 27 27 26 27 27 26 27 27 26 27 26 27 27 26 27 27 26 27 27 26 27 27 26 27 27 26 27 27 26 27 27 26 27 27 26 27 27 27 26 27 27 27 26 27 27 27 27 27 27 26 2 27 27 27 2 2 22 2 23 36 1100 1111111111111	650 1000 1500 Skin Friction (psf) ignore 650 1000 2000 1000 2000 1000 2000 1000 2000 1000 1000 1000 1500 15	9000 30000 36000 mate End Bearing Pressure (psf) ignore 19200 30000 19200 19200 mate End Bearing Pressure (psf) ignore 24000 25200 36000 mate End Bearing	325 500 750 Skin Friction (psf) ignore 325 500 1000 500 Skin Friction (psf) 450 750 750 750 750 750	4500 15000 18000 vable End Bearing Pressure (psf) ignore 9600 15000 9600 vable End Bearing Pressure (psf) ignore 12000 12600 18000 vable End Bearing Pressure (psf) ignore 12000 12600 18000 vable End Bearing Pressure (psf) ignore 12000 120	- - - - - - - - - - - - - -	1500 5000 6000 itress Cohesion, c (psf) ignore 1500 3200 5000 3200 5000 3200 5000 3200 5000 3200 5000 3200 5000 3200 5000 3200 5000 6000	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500 1000 1000 1000 1000 1000 Static Lateral Subgrade Modulus, k (pci) 500 1000 1000 2000 Static Lateral	0.007 0.005 0.004 Strain ε ₅₀ - 0.005 0.005 0.005 0.005 0.007 0.005 0.005 0.005	0.9 2.5 7 MFAD Deformation Modulus E _p (ksi) 0.8 0.9 2 3.5 2 MFAD Deformation Modulus E _p (ksi) 1.5 2.5 2.9 7 MFAD
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25' TL Depth 0-2.5 2.5'-4' 4'-6' 6'-10' 10-25' TL Depth 0-4 4'-14' 14'-20' 20'-25' TL	12 42 100 10 Avg N 8 12 26 51 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 51 27 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 27 26 27 26 27 27 26 27 27 26 27 26 27 27 26 29 29 29 29 29 29 29 29	650 1000 1500 Ultir Skin Friction (psf) ignore 650 1000 2000 1000 2000 1000 2000 1000 2000 1000 2000 1000 1000 1000 1500 15	9000 30000 36000 mate End Bearing Pressure (psf) ignore 19200 30000 19200 19200 19200 19200 19200 19200 2000 192000 192000 192000 192000 1920000 1920000000000	325 500 750 Skin Friction (psf) ignore 325 500 1000 500 Skin Friction (psf) 450 750 750 750 750 750 750	4500 15000 18000 vable End Bearing Pressure (psf) ignore 9600 15000 9600 vable End Bearing Pressure (psf) ignore 12000 12600 18000 vable End Bearing Pressure (psf) ignore 12000 12600 18000 vable End Bearing Pressure (psf) ignore 12000 12600 18000 vable End Bearing Pressure (psf) ignore 12000 18000 vable End Bearing Pressure (psf) ignore 12000 12600 18000 vable End Bearing Pressure (psf) ignore 12000 12600 18000 Vable End Bearing Pressure (psf) ignore 12000 12600 18000 18000 Vable End Bearing Pressure (psf) ignore 12000 18000 18000 18000 12000 1800	- - - Total S Internal Angle of Friction ϕ (degrees) 30 - - - Total S Internal Angle of Friction ϕ (degrees) - - - Total S Internal Angle of Friction ϕ (degrees) - - - - - - - - - - - - -	1500 5000 6000 itress Cohesion, c (psf) ignore 1500 3200 5000 3200 3200 3200 5000 3200 32	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500 1000 1000 1000 1000 1000 1000 10	0.007 0.005 0.004 Strain ε ₅₀ - 0.005 0.005 0.005 0.005 0.007 0.005 0.005 0.005 0.005	0.9 2.5 7 MFAD Deformation Modulus E _p (ksi) 0.8 0.9 2 3.5 3.5 2 3.5 3.5 2 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5
Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25' Depth 0-2.5 2.5'-4' 4'-6' 6'-10' 10-25' TL Depth 0-4 4'-14' 14'-20' 20'-25' TL	12 42 100 10 Avg N 8 12 26 51 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 27 26 51 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 27 26 27 26 27 26 27 26 27 27 26 27 26 27 27 26 27 26 27 26 27 26 27 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 27 26 27 26 27 26 27 26 27 27 26 27 27 26 27 26 27 27 26 27 27 26 27 26 27 27 26 27 27 26 27 27 27 26 27 27 26 27 27 27 26 27 27 27 26 27 27 27 27 27 26 27 27 27 27 27 27 27 27	650 1000 1500 Ultir Skin Friction (psf) ignore 650 1000 2000 1000 2000 1000 2000 1000 10	9000 30000 36000 mate End Bearing Pressure (psf) ignore 19200 30000 19200 19200 19200 mate End Bearing Pressure (psf) ignore 24000 25200 36000 mate End Bearing Pressure (psf)	325 500 750 Allov Skin Friction (psf) ignore 325 500 1000 500 Skin Friction (psf) 450 750 750 750 750 750 750 750 750 750 7	4500 15000 18000 vable End Bearing Pressure (psf) ignore 9600 15000 9600 vable End Bearing Pressure (psf) ignore 12000 12600 18000 vable End Bearing Pressure (psf) ignore 12000 12600 18000 vable End Bearing Pressure (psf) ignore 12000 12600 18000 vable End Bearing Pressure (psf) ignore 12000 1260 12600	- - - Total S Internal Angle of Friction ϕ (degrees) 30 - - - Total S Internal Angle of Friction ϕ (degrees) - - - Total S Internal Angle of Friction ϕ (degrees) - - - - - - - - - - - - -	1500 5000 6000 itress Cohesion, c (psf) ignore 1500 3200 5000 3200 5000 3200 5000 3200 5000 3200 itress Cohesion, c (psf) 2200 4000 4200 6000 itress Cohesion, c (psf)	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500 1000 1000 1000 1000 1000 1000 10	0.007 0.005 0.004 Strain ε ₅₀ 0.007 0.005 0.005 0.005 0.005 0.005 0.005 0.005 0.005	0.9 2.5 7 MFAD Deformation Modulus E _p (ksi) 0.8 0.9 2 3.5 2 MFAD Deformation Modulus E _p (ksi) 1.5 2.5 2.9 7 MFAD Deformation Modulus E _p (ksi)
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25' Depth 0-2.5 2.5'-4' 4'-6' 6'-10' 10-25' TL - 0-4 4'-14' 14'-20' 20'-25' TL - 0-4 4'-14' 14'-20' 20'-25'	12 42 100 10 Avg N 8 12 26 51 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 51 27 51 27 51 27 51 27 51 27 51 27 51 27 51 27 51 27 51 27 51 27 51 27 51 27 51 27 51 27 51 27 51 27 51 27 51 27 51 27 51 27 5151 51 51515151515151515151	650 1000 1500 Ultin Friction (psf) ignore 650 1000 2000 1000 2000 1000 2000 1000 10	9000 30000 36000 mate End Bearing Pressure (psf) ignore 19200 30000 19200 19200 19200 mate End Bearing Pressure (psf) ignore 24000 25200 36000 mate End Bearing Pressure (psf)	325 500 750 Allov Skin Friction (psf) ignore 325 500 1000 500 500 Skin Friction (psf) 450 750 750 750 750 750 750 750 750 750 7	4500 15000 18000 vable End Bearing Pressure (psf) ignore 9600 15000 9600 vable End Bearing Pressure (psf) ignore 12000 12600 18000 vable End Bearing Pressure (psf) ignore 12000 12600 18000 vable End Bearing Pressure (psf) ignore 12000 12600 18000 12600 18000 18000 18000 18000 15000 126	- - - - - - - - - - - - - -	1500 5000 6000 itress Cohesion, c (psf) ignore 1500 3200 5000 3200 3200 3200 3200 3200 3	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500 1000 1000 1000 1000 1000 1000 10	0.007 0.005 0.004 Strain ε ₅₀ - 0.007 0.005 0.005 0.005 0.005 0.007 0.005 0.007 0.005 0.007	0.9 2.5 7 MFAD Deformation Modulus E _D (ksi) 0.8 0.9 2 3.5 2 MFAD Deformation Modulus E _D (ksi) 1.5 2.5 2.9 7 MFAD Deformation Modulus E _D (ksi)
Sandy ML-CL Sandy ML-CL Sandy ML-CL Boring Soil Sandy ML-CL Sandy ML-CL	4'-8' 8'-18' 18'-25' Depth 0-2.5 2.5'-4' 4'-6' 6'-10' 10-25' TL - 0-4 4'-14' 14'-20' 20'-25' TL - 20'-25' TL - Depth 0-4 4'-14'	12 42 100 10 Avg N 8 12 26 51 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 27 26 51 29 26 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 20 51 51 51 51 51 51 51 51	650 1000 1500 Ultin Friction (psf) ignore 650 1000 2000 1000 2000 1000 2000 1000 2000 1000 2000 1000 1000 2000 1000000	9000 30000 36000 mate End Bearing Pressure (psf) ignore 19200 30000 19200 19200 mate End Bearing Pressure (psf) ignore 24000 25200 36000 mate End Bearing Pressure (psf) ignore 24000 25200 36000	325 500 750 Skin Friction (psf) ignore 325 500 1000 500 500 Skin Friction (psf) 450 750 750 750 750 750 750 750 750 750 7	4500 15000 18000 vable End Bearing Pressure (psf) ignore 9600 15000 9600 vable End Bearing Pressure (psf) ignore 12000 12600 18000 vable End Bearing Pressure (psf) ignore 12000 12600 18000 vable End Bearing Pressure (psf) ignore 12000 12600 18000 18000 18000 18000 15000 12000 1000	- - - - - - - - - - - - - -	1500 5000 6000 itress Cohesion, c (psf) 1500 3200 5000 3200 3200 3200 3200 3200 3	500 1000 2000 Static Lateral Subgrade Modulus, k (pci) 25 500 1000 1000 1000 1000 3tatic Lateral Subgrade Modulus, k (pci) 2000 3tatic Lateral Subgrade Modulus, k (pci) 2000	0.007 0.005 0.004 Strain ε ₅₀ 0.007 0.005 0.005 0.005 0.005 0.007 0.005 0.007 0.005 0.007 0.005 0.005 0.007 0.005 0	0.9 2.5 7 MFAD Deformation Modulus E _D (ksi) 0.8 0.9 2 3.5 2 MFAD Deformation Modulus E _D (ksi) 1.5 2.5 2.9 7 MFAD Deformation Modulus E _D (ksi) 0.8 5

				Rock Mass Rating (RMR76)							MFAD
Rock	Depth	qu (ksi)	RQD	qu (ksi)	RQD	Jt Space	Jt Cond	GW	Orient	RMR76	Deformation
Shale	10'-25'	1.5	0	1	3	5	0	7	-7	9	42
Boring	TL-32		Ultimate		Allowable		Total Stress		Static		
				End		End	Internal		Lateral		MFAD
			Skin	Bearing	Skin	Bearing	Angle of		Subgrade		Deformation
			Friction	Pressure	Friction	Pressure	Friction ϕ	Cohesion,	Modulus,		Modulus E _D
Soil	Depth	Avg N	(psf)	(psf)	(psf)	(psf)	(degrees)	c (psf)	k (pci)	Strain $\epsilon_{\scriptscriptstyle 50}$	(ksi)
Sandy ML	0-4	15	800	ignore	400	ignore	-	1800	500	0.007	1.5
Sandy ML-CL	4'-8'	23	950	12000	475	6000	40	3000	1000	0.005	1.8
				Rock Mass Rating (RMR76)							MFAD
Rock	Depth	qu (ksi)	RQD	qu (ksi)	RQD	Jt Space	Jt Cond	GW	Orient	RMR76	Deformation
W. Sandstone	8-13'	1.5	0	1	3	5	0	7	-7	9	42
Sandstone	13-25'	3	34	4	3	5	0	7	-7	46	1016
Boring	TL	-33	Ulti	mate	ate Allowable			Total Stress Sta			
				End		End	Internal		Lateral		MFAD
			Skin	Bearing	Skin	Bearing	Angle of		Subgrade		Deformation
			Friction	Pressure	Friction	Pressure	Friction o	Cohesion,	Modulus.		Modulus E _D
Soil	Depth	Avg N	(psf)	(psf)	(psf)	(psf)	(degrees)	c (psf)	k (pci)	Strain ϵ_{50}	(ksi)
Sandy ML	0-4	10	800	ignore	400	ignore	-	1200	500	0.007	0.7
Sandy CL	4'-12'	29	1000	19200	500	9600	-	3200	1000	0.005	2
Sandy CL	12'-25'	55	2000	30000	1000	15000	-	5000	1000	0.005	4
Boring	TL-43		Ultimate		Allowable		Total Stress		Static		
				End		End	Internal		Lateral		MFAD
			Skin	Bearing	Skin	Bearing	Angle of		Subgrade		Deformation
			Friction	Pressure	Friction	Pressure	Friction 	Cohesion,	Modulus,		Modulus E _D
Soil	Depth	Avg N	(psf)	(psf)	(psf)	(psf)	(degrees)	c (psf)	k (pci)	Strain ϵ_{50}	(ksi)
Sandy ML-CL	0-4	13	700	ignore	350	ignore	-	1500	500	0.007	0.9
Sandy CL	4'-17.5'	62	1500	12000	750	6000	-	5500	2000	0.004	5
				Rock Mass Rating (RMR76)						MFAD	
Rock	Depth	qu (ksi)	RQD	qu (ksi)	RQD	Jt Space	Jt Cond	GW	Orient	RMR76	Deformation
Sandstone	17.5-25'	1.5	0	1	3	5	0	7	-7	9	42